

2024 UPDATE Winneshiek Hydrologic Monitoring System Plan

Winneshiek County Landfill Decorah, Iowa

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Certification

Certification page (PE or groundwater scientist signature) **113.10(1)"d"**

For the purposes of this rule, a "qualified groundwater scientist" means a scientist or an engineer who has received a baccalaureate or postgraduate degree in the natural sciences or engineering and has sufficient training and experience in groundwater hydrology and related fields demonstrated by state registration, professional certifications, or completion of accredited university programs that enable that individual to make sound professional judgments regarding groundwater monitoring, contaminant fate and transport, and corrective action.

1 Introduction and Winneshiek County Landfill Site Background

This report presents the Hydrologic Monitoring System Plan (HMSP) for Winneshiek County Landfill. The HMSP has been designed to comply with IAC 567-113.10(455B). The following sections comprise the report:

- Section 1 Introduction and Winneshiek County Landfill Site Background
- Section 2 Conceptual Site Models
- Section 3 Theoretical Release Evaluation
- Section 4 Groundwater Monitoring Points
- Section 5 Semi-Annual Groundwater Measurements
- Section 6 Groundwater Sampling and Analysis
- Section 7 Detection Monitoring Program
- Section 8 Assessment Monitoring Program
- Section 9 Assessment of Corrective Measures
- Section 10 Annual Groundwater Quality Reports
- Section 11 Monitoring Well Maintenance and Performance Evaluation Plan

1.1 Winneshiek County Landfill Location

The Winneshiek County Landfill is in rural Winneshiek County approximately 5 miles southeast of the City of Decorah, Iowa, in the SE $\frac{1}{4}$ of Section 9 and the NE $\frac{1}{4}$ of Section 16, T 97 N, R 7 W (Figure 1). Winneshiek County Landfill was opened in 1974 by Nishna Sanitary Services as a privately owned and operated facility. In 1991, the Winneshiek County Solid Waste Agency purchased the facility. The facility accepts non-hazardous municipal solid waste.

Winneshiek County Landfill History and Construction

At the Winneshiek County Landfill, the different areas (cells) were constructed over time due to different regulations and, as a result, there are differences in the liner, leachate-collection, and landfill gas systems. In addition, construction, excavation, filling, and capping activities have locally altered the surficial groundwater system, affecting present day shallow groundwater monitoring at the site.

The present landfill consists of a closed area and an active composite lined cell C5 EXP, see (Figure 2).

Area 1 was constructed in 1974 without a liner or leachate collection system, but a leachate toe drain (Figure 2 – West Toe Drain) was installed around the east, west and north perimeter in 1993. Area 1 was permanently closed in 2017 with a soil cover along the 4:1 slope and a composite cover over the lesser slope.

Area 2 was developed beginning in 1982 and was used until 1994, except for some construction and demolition (C&D) disposal that continued in a small portion of Area 2 until 2000-2002. Area 2 was constructed without a liner or leachate collection system, but a leachate toe drain (Figure 2 – West Toe Drain) was installed around the west perimeter in 1993 and a landfill gas collection trench along the south side. Area 2 was permanently closed in 2017 with a soil cover along the 4:1 slope and a composite cover over the lesser slope.

Development of Area 3 began in 1988 and continued until 1994. Area 3 was constructed without a liner or leachate collection system, but it has a leachate toe drain (Figure 2 – Central Toe Drain) along the eastern side and a landfill gas collection trench along the south side. Area 3 was permanently closed in 2017 with a soil cover along the 4:1 slope and a composite cover over the lesser slope.

Development of Cell 4 began in 1994 and continued until 2017. It is constructed with a clay liner and leachate collection system. An abutment liner was installed across Cell 4 in 2017. The side slopes have temporary cover.

Development of Cell 5 began in 1996 and is incorporated into C5 EXP. It is constructed with a composite liner and leachate collection system.

Development of Cell 5 Expansion (C5 EXP) Area began in 2016 and continues to the present. It is a small, 1-acre cell located between the east side of Area 1 and the north side of Cell 5. It is constructed with a composite liner and leachate collection system.

C5 EXP is the active cell. It is constructed with a composite liner and leachate collection system. This cell consists of the following:

- Abutment Liner
	- East Slope of Area 1
	- ─ East Slope of Area 3
	- ─ Cell 4
- Cell 5
- Cell 5 Expansion Area

Areas 1, 2, and 3 have a passive gas venting system, and there is a landfill gas collection trench with gas vents south of Areas 2 and 3. The passive venting system will be continued over the active area as it is closed.

Winneshiek County Landfill Geology and Surface Drainage

Winneshiek County Landfill is located on a generally eastward sloping upland that is part of an interfluve bordered on the east by an unnamed, northward flowing tributary to the Trout River and on the northwest by a different unnamed, northward flowing drainageway that drains to the Trout River (Figure 1). In this part of Winneshiek County, thin Quaternary-age deposits overlie Ordovician-age bedrock.

In uplands, the thin Quaternary deposits include loess overlying either remnants of Pre-Illinoian-age glacial diamicton ("till") that locally contains isolated lenses of fluvioglacial material or Quaternary-age colluvium. On side slopes, the loess overlies either Quaternary-age colluvium or, where the colluvium is absent, bedrock. In lowlands, the thin Quaternary-age deposits consist of fine-grained alluvium. In portions of the site area, such as east of Area 4, landfill construction activities have removed the Quaternary deposits, and fine-grained, construction-derived soil fill associated with the landfill road overlies Ordovician-age Maquoketa Formation bedrock. Fine-grained, construction-derived soil fill overlies loess and Ordovicianage Galena Group Dubuque Formation bedrock east of Area 5.

The Quaternary-age deposits overlie a gently dipping Ordovician-age bedrock sequence that consists of the upper and lower Elgin Member of the Maquoketa Formation and the underlying Galena Group Dubuque Formation. In parts of the northeastern portion of the site, the Maquoketa Formation is eroded away and absent. In that area, the Galena Group Dubuque Formation is the uppermost bedrock unit.

Groundwater is monitored in one water-bearing zone: (1) the shallow groundwater measured by the water table, and (2) the deep groundwater measured in the Ordovician-age upper and lower Elgin Member of the Maquoketa Formation and, where the Maquoketa is absent, the Ordovician-age Galena Group Dubuque Formation. Shallow wells are screened in different geologic units because the water table occurs in successively different stratigraphic horizons downslope. The water table and bedrock groundwater are

interconnected and act as one water-bearing zone transitioning to only water table on the eastside of the landfill with groundwater discharging to tributary of Trout River.

2 Conceptual Site Models

Conceptual Site Models (CSMs) of a hydrogeologic system provide an overall understanding of groundwater and contaminant flow beneath landfills based on an interpretation of available regional and site hydrogeologic data. CSMs are based on consideration of site area topography, landfill construction, site geology (including geologic units beneath the landfill area and their thickness, extent, and properties), the water table in the uppermost sediments, and groundwater flow in the uppermost bedrock.

The Winneshiek County Landfill consists of five different areas that are regulated by the Iowa DNR (Figure 2). These differ in topography, landfill construction, and water table relationships. Consequently, separate CSMs will be presented for these different settings: Area 1, Areas 2 and 3, Cell 4, and C5 EXP.

Common to the different landfill areas are the topography, site geology, water table flow across the site, and groundwater flow in the uppermost bedrock. The landfill areas differ in location and landfill construction.

The common topography, geology, site water table, and bedrock groundwater flow conditions will be discussed first, then separate conceptual models will be developed for the various areas based on the differences in landfill construction.

2.1 Common Conditions Across the Winneshiek County Landfill

2.1.1 Topography

The Winneshiek County Landfill lies within the Paleozoic Plateau region of northeastern Iowa. Deep valleys, numerous rock outcroppings, high bluffs, and an angular step relief characterize the Paleozoic Plateau's terrain where bedrock is the primary control on topography (Horick 1989; Prior 1991).

The landfill is located on a generally eastward sloping upland that is part of an interfluve bordered on the east by an unnamed, northward flowing tributary to the Trout River and on the northwest by an unnamed, northward flowing drainageway that drains to the Trout River. Figure 1 shows the topography of the site area on a USGS 7.5 Minute Topographic Map prior to landfill development, and Figure 3 shows the topography of the landfill site in 2019 inside of the C5 EXP boundary and IDNR LiDAR dated 2010 outside the boundary. Total relief prior to landfill construction was on the order of 80 feet. The landfill was constructed sloping to the east-northeast from a hillslope summit in the southwestern part of the site.

2.1.2 Site Geology

The geology of site wells is included with the boring logs and monitoring well/piezometer documentation forms (Iowa DNR Form 542-1277) presented in Appendix A. Boring and well locations are shown on Figure 4. Geologic cross-sections were created using site stratigraphy logged and described from all site borings. Figure 5 presents the locations of the geologic cross-sections. Geologic cross-sections are presented in Figures 6 through 20.

As noted in Section 1, in uplands, the thin Quaternary deposits include loess overlying either remnants of Pre-Illinoian-age glacial diamicton ("till") that locally contains isolated lenses of fluvioglacial material or Quaternary-age colluvium. On side slopes, the loess overlies either Quaternary-age colluvium or, where the colluvium is absent, bedrock. In lowlands, the thin Quaternary-age deposits consist of fine-grained alluvium. In portions of the site area, such as east of Cell 4, landfill construction activities have removed the Quaternary deposits along the road, and fine-grained, construction-derived soil fill overlies Ordovicianage Maquoketa Formation bedrock. Fine-grained, construction-derived soil fill overlies loess and Ordovician-age Galena Group Dubuque Formation bedrock east of C 5 EXP.

Beneath the Quaternary-age deposits is a gently dipping Ordovician-age bedrock sequence that consists of the upper and lower Elgin Member of the Maquoketa Formation and the underlying Galena Group Dubuque Formation. The contact between the Maquoketa Formation and the underlying Galena Group Dubuque Formation is an unconformity (an uneven erosion surface). In parts of the northeastern portion of the site, the Maquoketa Formation is believed eroded away and absent. In that area, the Dubuque Formation is the uppermost bedrock unit.

2.1.3 Water Table

Figure 21 presents the water table in May 2023 which is representative of water table conditions at the site. Different geologic units are present while traversing downslope, west to east, from the uplands toward the Trout River at the Winneshiek County Landfill, and the water table crosscuts these units which include Quaternary-age loess, Pre-Illinoian-age glacial deposits, Quaternary-age colluvium, Maquoketa Formation shale, and Galena Group Dubuque Formation limestone. In the Trout River lowlands east of the landfill, the water table is in the Quaternary-age alluvium or top of weathered Galena Group Dubuque Formation limestone. Over the site, there is a groundwater divide present in the water table. On the east part of the site or divide, shallow water table flow is downslope to the northeast toward the tributary of the Trout River; along the western and northwestern part of the site adjacent to Area 1 and Area 2, shallow water table flow is to the northwest toward a surface water drainageway and then to an unnamed tributary of the Trout River. Landfill areas at the Winneshiek County Landfill were constructed above the water table.

2.1.4 Deep Groundwater Flow in the Uppermost Bedrock

Figure 22 presents groundwater levels measured in bedrock for May 2023 which is representative of bedrock groundwater conditions at the site. The uppermost bedrock beneath the site is a gently dipping Ordovician-age bedrock sequence that consists of the upper and lower Elgin Member of the Maquoketa Formation and the underlying Galena Group Dubuque Formation. The contact between the Maquoketa Formation and the underlying Dubuque Formation is an unconformity (an uneven erosion surface). The Maquoketa Formation is composed of significant thicknesses of shale and claystone, acting as a regional aquitard. The Galena Group, composed of limestone and dolomite, is part of a regional Galena aquifer system. In parts of the northeastern portion of the site, the Maquoketa Formation is largely eroded away and absent. In that area, the Dubuque Formation is the uppermost bedrock unit.

Groundwater flow within the Maquoketa Formation aquitard is primarily downward, to the top of the Galena Group and then groundwater flow in the Galena Group aquifer is primarily horizontal to the northeast. Regional groundwater flow in the Galena Group is expected to be primarily east toward the Trout River, the regional groundwater discharge point.

2.1.5 Groundwater Geochemistry

Winneshiek groundwater monitoring wells, surface water, underdrains, and leachate manholes were sampled for major anions and cations in June 2023. The major ionic species in most natural waters are Na⁺, K⁺, Ca⁺², Mg⁺², Cl⁻, CO₃⁻², HCO₃ and SO₄⁻². The sample results were used to create piper and stiff diagrams.

Figures 23, 24 and 25 are a piper diagrams of anion and cation sample data for the Winneshiek groundwater, surface water, underdrain, and manhole monitoring points. The plots demonstrate different hydrochemical facies related to groundwater flow through different lithologies. On the west side of the landfill and mapped water table divide, wells MW-1, MW-2R, MW-3, MW-34A, MW-37A, MW-41A, MW-4, and MW-4B are similar in geochemistry because groundwater flows from upgradient MW-1 toward downgradient MW-41A (Figure 23). On the east side of the landfill, groundwater wells MW-7A, MW-24A, MW-25A, MW-29A, MW-31A, MW-35, MW-38A, MW-39A, MW-40A, MW-42A, MW-43A, MW-44A, and MW101 and surface water SW-1, SW-2 and SW-5 are similar in geochemistry because groundwater flows from MW-19 toward MW-40A, on the east side of water table divide and represent another hydrochemical facies (Figure 24). Last, bedrock wells MW-19, MW-22 and MW-35 located on the east side of the landfill, represents groundwater flow from Cell 4 towards the groundwater discharge to creek as reflected by surface water SW-1, SW-2 and SW-5 (Figure 25). The eochemistry confirms that shallow and deep groundwater are interconnected and combine as grondwater flows from west (MW-1 and MW-19) to east (MW-35 and MW-40A).

In terms of cations, most of the groundwater monitoring wells are calcium type. The exceptions are well MW-31A and manhole MH-4-2 are sodium or potassium type. In terms of anions, most of the groundwater monitoring wells and manholes are bicarbonate type. The exception is monitoring well MW-11 which is sulfate type. The trilinear plot shows a relationship between manhole MH-4-2 and downgradient monitoring well MW-31A, with these two sample points plotting close to each other.

Figures 26, 27 and 28 are a stiff diagrams of anion and cation sample data for the Winneshiek groundwater, surface water, underdrain, and manhole monitoring points. The plots are similar to the piper diagram, demonstrating different hydrochemical facies related to groundwater that flows through different lithologies. The stiff diagram supports the water table well groupings presented above.

The stiff plot shows a relationship between downgradient monitoring wells MW-31A, MW-42A and MW-43A and surface water at SW-5. The polygonal shape and area show a decrease in ion concentrations as groundwater flows from MW-31A downgradient to MW-42A and then discharging as measured at SW-5. This affirms that concentrations are attenuating as groundwater moves towards and discharges to the tributary of the Trout River.

2.2 Area 1 Site Conditions and Conceptual Site Model

2.2.1 Landfill Construction

Waste disposal at the Winneshiek County Landfill began in 1974 with the development of Area 1. Area 1 is now closed and has a vegetated intermediate cover, installed in 1994.

Base grade elevations for Area 1 are shown on Figure 29. The base grades were set above the water table, and base grade elevations decline downslope to the east-northeast from an elevation of about 1,172 feet to 1,160 feet above mean sea level (MSL). Area 1 was constructed without a liner or leachate collection system, but a leachate toe drain was installed around the western, northern, and eastern perimeter in 1993. Area 1 has a passive landfill gas system.

2.2.2 Conceptual Site Model for Area 1

The CSM for Area 1 is based on the site topography, landfill construction, underlying site geology, water table conditions, and groundwater flow in the uppermost bedrock units discussed in the previous sections.

The CSM for Area 1 is depicted on site cross-sections in Figures 6, 7, 8, and 15. Most of the precipitation at the site is intercepted by the landfill cap and the surface water drainage system and then routed to drainageways draining off-site to the west, north, and east. A portion of the precipitation is lost to the atmosphere by evapotranspiration. A limited amount of precipitation infiltrates to the water table in the Quaternary-age loess and fine-grained Pre-Illinoian-age glacial diamicton.

The landfill has been constructed on a hillslope with base grades that follow the natural slope to the eastnortheast. Area 1 does not have a liner or leachate control system, but leachate flow is predominantly downslope along the base grade to the east-northeast. A leachate toe drain was constructed around the western, northern, and eastern perimeter of Area 1 (Figure 4) to intercept leachate. Leachate intercepted by the toe drains is directed to the leachate lagoon where it is evaporated and re-circulated back into the landfill cells. Any constituent not intercepted by the leachate toe drain is expected to slowly migrate downward to the water table. A water table divide splits Area 1 into two water table flow directions; the east half of Area 1 shallow groundwater flows toward the east-northeast; the west half of Area 1 shallow groundwater flows toward the west-northwest. Shallow groundwater quality is monitored by downgradient water table wells. Bedrock groundwater flow is east toward the Trout River. Deep groundwater quality is monitored by downgradient bedrock wells. The groundwater monitoring points for the Winneshiek County Landfill (see Section 4) have been placed in accordance with this Conceptual Site Model.

2.3 Area 2 and Area 3 Site Conditions and Conceptual Site Model

2.3.1 Landfill Construction

Area 2 was developed beginning in about 1982 and was used until 1994, except for some C&D disposal that continued in a small portion of Area 2 until 2000-2002. Development of Area 3 began in about 1988 and continued until 1994. Both Area 2 and Area 3 are closed. Area 2 and Area 3 have a vegetated intermediate cover, installed in 1994. Area 2 and Area 3 have a passive gas venting system, and there is a landfill gas collection trench with gas vents south of Areas 2 and 3.

Base grade elevations for Area 2 and Area 3 are shown on Figure 29. Base grades follow the natural slope to the north-northeast, and base grade elevations range from about 1,200 feet MSL to 1,160 feet MSL.

Area 2 and Area 3 were constructed above the water table and have no liner or leachate collection system, but a leachate toe drain is installed along the western perimeter of Area 2 and the eastern perimeter of Area 3 (Figure 4).

2.3.2 Conceptual Site Model for Area 2 and Area 3

The CSM for Area 2 and Area 3 is based on the site topography, landfill construction, underlying site geology, water table conditions, and groundwater flow in the uppermost bedrock units discussed in the previous sections.

The CSM for Area 2 and Area 3 is depicted on a site cross-section on Figures 9, 10, 11, 12, 13, 14, and 15. Most of the precipitation at the site is intercepted by the landfill cap and the surface water drainage system and routed to drainageways draining off-site to the north, south, east, and west. A portion of the precipitation is lost to the atmosphere by evapotranspiration. A limited amount of precipitation infiltrates to the water table in the Quaternary-age loess, fine-grained Pre-Illinoian-age glacial diamicton, and Maquoketa Formation claystone.

The landfill has been constructed on a hillslope with base grades that follow the natural slope to the northnortheast. Areas 2 and 3 do not have a liner or leachate control system, but leachate flow is predominantly downslope along the base grade to the east-northeast. Leachate toe drains are constructed along the western perimeter of Area 2 and the eastern perimeter of Area 3 to intercept any leachate. Leachate intercepted by the toe drains is directed to the leachate lagoon where it is evaporated and re-circulated back into the landfill cells. Any constituent not intercepted by the leachate toe drain is expected to slowly migrate downward to the water table. A water table divide splits Areas 2 and 3 into two water table flow directions; the east half of Area 3 shallow groundwater flows toward the northeast; the west half of Area 2 shallow groundwater flows toward the west-northwest. Shallow groundwater quality is monitored by downgradient water table wells. Bedrock groundwater flow is east toward the Trout River. Deep groundwater quality is monitored by downgradient bedrock wells. The groundwater monitoring points for Areas 2 and 3 (refer to Section 4) have been placed in accordance with this Conceptual Site Model.

2.4 Cell 4 Site Conditions and Conceptual Site Model

2.4.1 Landfill Construction

Development of Cell 4 began in 1994 and is closed. It is constructed above the water table with a clay liner and leachate collection system.

The base grade elevations for Cell 4 are shown on Figure 29. Base grades follow the natural slope to the northeast, and base grade elevations range from about 1,184 feet MSL to 1,162 feet MSL.

Cell 4 is not active and has a passive landfill gas system installed.

2.4.2 Conceptual Site Model for Cell 4

The CSM for Cell 4 is based on the site topography, landfill construction, underlying site geology, water table conditions, and groundwater flow in the uppermost bedrock units discussed in the previous sections.

The CSM for Cell 4 is depicted on a site cross-section on Figures 10, 11, 12, 16, and 17. Most of the precipitation at the site is intercepted by the landfill cap and the surface water drainage system and routed to drainageways draining off-site to the east. A portion of the precipitation is lost to the atmosphere by evapotranspiration. A limited amount of precipitation infiltrates to the water table in the Quaternary-age loess and Ordovician-age Maquoketa Formation.

The landfill has been constructed on a hillslope with base grades that follow the natural slope to the east and northeast. Cell 4 has a clay liner and leachate control system, and leachate flow along the liner is downslope to the northeast where it is collected at the leachate lagoon. Any constituent not intercepted by the leachate control system is expected to slowly migrate downward to the water table. Water table and bedrock flow for Cell 4 is predominantly to the east, toward the Trout River. Shallow groundwater quality is monitored by downgradient water table wells, while deep groundwater quality is monitored by downgradient bedrock wells. The groundwater monitoring points for Cell 4 (refer to Section 4) have been placed in accordance with this Conceptual Site Model.

2.5 C5 EXP Site Conditions and Conceptual Site Model

2.5.1 Landfill Construction

Development of Cell 5 began in 1996 and continues to the present. The area is now referred to as C5 EXP but also includes the abutment liner as described in Section 1.2. It is constructed above the water table with a composite liner and leachate collection system.

The base grade elevations for C5 EXP are shown on Figure 29. Base grades follow the natural slope to the northeast, and base grade elevations range from about 1,160 feet to 1,122 feet MSL.

Cell 5 EXP is still active and will have a landfill gas system installed when it is closed.

2.5.2 Conceptual Site Model for C5 EXP

The CSM for C5 EXP is based on the site topography, landfill construction, underlying site geology, water table conditions, and groundwater flow in the uppermost bedrock units discussed in the previous sections.

The CSM for C5 EXP is depicted on site cross-sections in Figures 7, 8, 9, 16, 17, 18, 19, and 20. Most of the precipitation at the site is intercepted by the landfill cap and the surface water drainage system and routed to drainageways draining off-site to the north and east. A portion of the precipitation is lost to the atmosphere by evapotranspiration. A limited amount of precipitation infiltrates to the water table in the Quaternary-age loess and Ordovician-age Maquoketa Formation claystone.

The landfill has been constructed on a hillslope with base grades that follow the natural slope to the northeast. C5 EXP has a composite liner and leachate control system, and leachate flow is downslope along the liner to the northeast where it is collected at the leachate lagoon. Two sections of C5 EXP have a groundwater drainage layer which intercepts water table groundwater lowering groundwater levels below the landfill base grade. Any constituent not intercepted by the leachate control system, or the groundwater drainage layer is expected to slowly migrate downward to the water table. Water table and bedrock flow for C5 EXP is predominantly to the east, toward the Trout River. Shallow groundwater quality is monitored by downgradient water table wells, while deep groundwater quality is monitored by downgradient bedrock wells. The groundwater monitoring points for C5 EXP (see Section 4) have been placed in accordance with this Conceptual Site Model.

2.6 Reference

Horick, P. J. 1989. Water Resources of Northwest Iowa, Iowa Department of Natural Resources, Water Atlas Number 8, 145 p.

Prior, J.C., 1991. Landforms of Iowa: University of Iowa Press, Iowa City, 153 p.

3 Theoretical Release Evaluation

The theoretical release evaluation for the Winneshiek County Landfill is based upon the landfill area conceptual site models (CSM) discussed in Sections 2.2 through 2.5, on an understanding of landfill construction (base grades slope and direction, presence or absence of leachate collection systems, landfill cap, relationship to the water table), regional and site-specific geologic and hydrogeologic conditions and groundwater flow modeling results.

All the Landfill areas at the Winneshiek County Landfill were constructed above the water table in the surficial sediments and were constructed to promote leachate drainage downslope. Area 1 does not have a liner or leachate control system, but leachate flow is predominantly downslope along the base grade to the west-northwest. A leachate toe drain is constructed around the western, northern, and eastern perimeters of Area 1 (Figure 2) to intercept any leachate. Area 2 does have the leachate toe drain for leachate control, with leachate flow predominantly downslope along the base grade to the west-northwest and to the toe drain. Area 3 does have the leachate toe drain for leachate control, with leachate flow predominantly downslope along the base grade to the east-northeast and to the toe drain. Leachate toe drains are constructed along the western perimeter of Area 2 and the eastern perimeter of Area 3 to intercept any leachate. A release of leachate not intercepted by the leachate toe drains for Areas 1, 2, and 3 is expected to slowly migrate downward to the water table.

Cell 4 has a clay liner and leachate control system, and leachate flow along the liner is downslope to the east-northeast. C5 EXP has a composite liner and leachate control system, and leachate flow is downslope along the liner to the east-northeast. A release of leachate not intercepted by the leachate control system for Cells 4 and C5 EXP is expected to slowly migrate downward to the water table.

Because the Landfill areas and cells were constructed above the water table, the water table is considered the primary pathway for any release from the Landfill. Water table gradients promote downslope groundwater flow toward shallow Quaternary-age loess, Pre-Illinoian-age glacial deposits, Quaternary-age colluvium, and Maquoketa Formation shale downslope of the Landfill. Over most of the site, shallow water table flow is downslope to the northeast; along the western and northwestern part of the site, shallow water table flow is to the west-northwest. Shallow groundwater quality is monitored by downgradient water table and bedrock monitoring wells. Surface water is monitored at upstream staff gauge SW-1 and downstream at staf gauges SW-2, SW-4 and SW-5. The groundwater monitoring points for the Winneshiek County Landfill (see Section 4) have been placed at approximately 300-foot spacings in accordance with the IAC 567 113.10(2)e(2).

A release of leachate not following the leachate collection system, leachate toe drains, and water table could potentially migrate slowly downward through the Ordovician-age Maquoketa Formation aquitard toward the Galena Group aquifer. Because of the hydraulic conductivity contrast between the surficial Quaternary-age loess, Pre-Illinoian-age glacial deposits, Quaternary-age colluvium, and lower hydraulic conductivity Ordovician-age Maquoketa Formation, the primary direction of groundwater flow is laterally within the water table, flowing downslope toward tributaries of the Trout River on the east and west sides of the landfill and the Trout River east of the landfill. The water table also occurs within top of bedrock on the east side of the landfill. Given that, downgradient bedrock aquifer wells are installed to provide information on groundwater head, groundwater quality, vertical gradients, and vertical flow as groundwater from the water table flows toward bedrock and, as such, bedrock will be monitored over time.

3.1 Theoretical Release Model

As part of the theoretical release modeling, groundwater flow was evaluated using GFLOW, a groundwater flow model. GFLOW was used to evaluate flow path lines from each of the five cells which comprise the Winneshiek County Landfill.

3.1.1 Groundwater Flow Model Construction

The conceptual model, numerical simulation code, and the numerical model construction details are described in this section.

3.1.1.1 Code Selection

A numerical groundwater flow model was selected as the method to simulate groundwater flow for the landfill area. The model is the mathematical representation of groundwater flow in an aquifer using a numerical analysis computer code. The computer code solves the governing systems of equations for groundwater flow.

The model, GFLOW (Haitijema, 2018), was chosen to simulate groundwater flow for the Winneshiek County Landfill theoretical release modeling. GFLOW uses an analytical element method to model groundwater flow. Analytic element methods use direct solutions to mathematical equations that represent hydrogeologic features in a groundwater flow model.

GFLOW was used to develop a regional-site model. GFLOW uses a stepwise approach that models steady-state groundwater flow in a single, heterogeneous aquifer. The GFLOW model was used for estimating aquifer parameters, setting boundary conditions, and provides theoretical release analyses.

3.1.1.2 GFLOW Regional-Site Model

A binary base map derived from USGS digital line graphs (DLG), and site CAD maps were imported directly into the modeling software to serve as the geographic basis and reference for the model domain. The units for the base map are in meters and geodetic reference is Universal Transverse Mercator (UTM) coordinate system, North American Datum (NAD), 1983, Zone 15. Units for groundwater modeling are in feet and days.

The GFLOW model domain (Figure 30) includes all major drainage basins in the vicinity of the project area, ranging from the unnamed tributary of the Trout River on the east side of site, the Trout River on the east model boundary, an unnamed tributary of the Trout River on the west side of the site, and Trout Creek on the west model boundary. The geometry of the model layer includes a bottom elevation of 1,000 ft above mean sea level (AMSL). The single layer represents a composite of the unconsolidated Quaternary sediments and weathered sedimentary bedrock.

The GFLOW model includes "far field" and "near field" line sinks. In the far field, streams and lakes are simulated with coarse line sinks having little or no leakage (resistance) between the surface water feature and the groundwater system. The purpose of simulating the far field is to have the model explicitly define the regional groundwater flow in the vicinity of the area of interest. The near field represents the area of interest and includes several of the streams adjacent to and including Trout River. Near-field streams are simulated using slightly more detailed line sinks with streambed leakage (resistance) to control groundwater/surface water interaction. The line sinks representing streams were assigned stages based on site data and USGS 7.5-minute quadrangle maps. Near field line sinks were assigned stream widths (ranging from 5 to 100 feet) based on field measurements and stream order.

Other input parameters to the GFLOW model include recharge, transmissivity, drains, and pumping wells. The transmissivity and recharge zones extend over both the near field and far field of the model. There are no large capacity pumping wells within the model boundary. Table 1, GFLOW Model Parameters, presents the settings and boundary conditions applied to the GFLOW model.

Table 1 Winneshiek GFLOW Model Input Parameters

3.1.1.3 Model Input Parameters

Aquifer Properties

The sediments above bedrock and weathered bedrock are the only geologic units in the groundwater model. Aquifer properties are based on field measurements collected during the site investigation for landfill permit. The aquifer properties required by GFLOW are aquifer thickness and hydraulic conductivity. Aquifer thickness was determined from boring logs presented in the hydrogeological investigation reports (JMM 1990; RUST 1993; RUST 1997; Earth Tech 2000) and from geologic cross-sections as described in Section 2. The aquifer thickness was set to 200 feet based on geologic cross-sections and the relief across the site. The bottom elevation of the model represents the groundwater discharge elevation at tributary of Trout River on the downgradient side of landfill area.

Hydraulic conductivity measurements were collected during the hydrogeological investigation at wells screened in the sediments above bedrock and bedrock monitoring wells. Hydraulic conductivity measurements were estimated by the slug test method.

Hydraulic conductivities estimated from slug tests conducted in wells screened in the loess/till/alluvial deposits range from approximately $2.1x10^{-5}$ to $9.6x10^{-3}$ cm/sec (0.06 to 27.2 ft/day). For the model, the hydraulic conductivity was varied up to one order of magnitude higher (faster) than the results of slug test analyses. The slug test method estimates hydraulic conductivity based on a small area surrounding the well, and studies indicate that slug tests are approximately one order of magnitude lower than pump tests conducted on the same aquifer.

Surface Water

Several surface water bodies were included in the groundwater model: Trout River and Trout Creek. The rivers and creeks were modeled as streams using line sinks with specified head and stream flow routing. Stream elevations were estimated based on USGS gauging stations and the staff gauges located at the site. Streambed width and depths were based on USGS gauging stations, USGS topographic maps, and site data. Streambed resistance is the thickness of the resistance layer between the surface water feature and the aquifer divided by the average vertical hydraulic conductivity of the resistance layer. Resistance values were calculated using site hydraulic conductivity data.

Recharge

Recharge is the portion of precipitation that does not become surface runoff, and percolates through the ground surface to the water table. The recharge was estimated using 6 inches/year.

Leachate Toe Drain

The leachate tow drain installed around Areas 1, 2, and 3 is not modeled. This conservative approach assumes the leachate toe drain does not exist in order to model natural groundwater flow directions preinstallation of the tow drain.

3.2 Model Results

3.2.1 Groundwater Elevations and Flow

The modeled water table is shown in Figure 32. In the site area, the groundwater flow system is closely approximated by the groundwater model. As measured at the site, groundwater flows toward an unnamed tributary of the Trout River on the northwest and the Trout River on the east. Surface water flow discharges to an unnamed tributary of the Trout River on the northwest and the Trout River on the east.

3.2.2 Model Calibration

The groundwater flow model was calibrated using a manual calibration approach whereby the model hydraulic parameters were optimized to minimize the difference between the model-predicted heads and stream flows and actual field measured heads and stream flows. The initial model input consisted of hydraulic parameters, recharge rates, and boundary conditions. Hydraulic conductivity, resistance and recharge rates were the parameters varied during the calibration. Specified head boundary conditions were also revised to achieve calibration. Model verification was also performed by using the calibrated hydraulic parameters and boundary conditions to perform a simulation of the conditions observed in August 2022.

3.2.2.1 Calibration Procedure

The calibration was performed by repeatedly running the model while varying the hydraulic conductivity, resistance, and recharge between each run, until the difference in model-predicted heads and surface water flows and field measured heads and surface water flows (the "residuals") were minimized. A calibration strategy using manual calibration was chosen because the high hydraulic gradients, thin saturated thicknesses, and high degree of variation in measured heads at the site. Use of an inverse model such as UCODE or PEST (automatic calibration) to check calibration and perform sensitivity analyses will be investigated during future modeling tasks.

Calibration parameter values were initially assigned to the model based on field measured values and, if measured values were not available, ranges of values for each hydrogeologic zone. These parameters were then adjusted between each model run to reduce the model residual. In many cases, physical constraints associated with the site determined the range over which calibration parameters could be varied.

3.2.2.2 Calibration Criteria

Due to the significant range of measured water table elevations, three statistical methods for analyzing the residuals were used, along with visual comparison of measured heads to model-predicted heads, to determine the calibration. The average error (AE), median error (ME), mean absolute error (MAE), the root mean square (RMS) error and sum of squared differences (SSD) were the statistical methods used to evaluate the model residual. The AE, which is the mean of all the residuals, indicates if the overall model response is high (a positive AE) or low (a negative AE) with respect to the measured values. The MAE, which is the mean of the absolute value of all the residuals, indicates the absolute average error in the model. The RMS error, which is the square root of the mean of the squared residuals, is usually represented as a percentage of the total head difference in the hydrologic system. The total head difference in the system is defined as the range of values observed in the measured heads. As described by Anderson, Woessner and Hunt (2015), if the percent RMS error is small, the errors represent only a small part of the overall model response. The goal of the calibration was to reduce model AE to less than 5 feet, RMS error to less than 10 feet, and individual residuals at each well to less than 5 feet.

3.2.2.3 Calibration Data

The model was calibrated to the water levels measured in 33 monitoring wells at the site on August 11, 2022. Calibration targets were randomly distributed across the model.

3.2.2.4 Calibration Results

Model calibration was performed until no significant reductions in the AE, ME, MAE, RMS, and SSD errors were being achieved with additional calibration simulations. Graphically, Figure 31 presents modeled head versus observed head. Targets aligning with the center 45-degree lines are those with no residual, i.e., exact calibration. Those targets falling above the centerline have simulated elevations greater than the target elevations. Those water levels falling below the centerline have simulated elevations less than the target elevations. In general, most of the targets for each layer plot along the 45-degree line which further demonstrates a reasonable calibration. A summary of the calibration statistics is tabulated in Table 2.

Figure 31 - Winneshiek County Landfill Modeled Head vs. Observed Head

3.3 Theoretical Release Particle Path Lines

As part of the theoretical release, particle tracking was conducted to assess the groundwater migration pathways and travel times under steady-state conditions at the site. Particles were tracked in steady-state flow domains, both forward and reverse, using August 2022 groundwater levels. Particles were introduced into the flow domain in six areas of the model: Area 1, Area 2, Area 3, Cell 4, Cell 5, and Cell 5 EXP.

The particle tracking was conducted using GFLOW (Haitijema 2018). The GFLOW tracking timestep was set to 1 year (365 days), with a maximum of 1000 years (365,000 days) for forward tracking and 1000 years for reverse tracking. Particle tracking tic marks were set to 50-yr (18,250 days) increments. Recharge was treated as a distributed source in GFLOW, and all particles were stopped when they entered strong internal sinks. Particles which encountered weak sinks passed through.

3.3.1 Theoretical Release: Area 1

Figure 33 presents the results of the theoretical release from closed landfill Area 1. Particles were placed within the footprint of Area 1 and at upgradient well locations corresponding to MW-1, MW-37A, and MW-19R. The results of the particle path line modeling show the groundwater flow paths from Area 1. The flow paths show that there is a groundwater divide where the west half of Area 1 flows toward the northwest, and the east half of Area 1 flows toward the northeast. Wells MW-1 and MW-37A are upgradient of Area 1 on the west side of groundwater divide. There are no upgradient wells for the east side of Area 1. Area 1 is monitored by downgradient water table wells MW-41A, MWII-2, MW-4, MW-4B, MW-34A, and MW-45A, and by bedrock well MW-18. Travel times from Area 1 towards the northwest drainageway are approximately 300-years.

3.3.2 Theoretical Release: Area 2

Figure 34 presents the results of the theoretical release from closed landfill Area 2. Particles were placed within the footprint of Area 2 and at upgradient well locations corresponding to MW-1, MW-37A, and MW-19R. The results of the particle path line modeling show the groundwater flow paths from Area 2. The flow paths show that there is a groundwater divide where the north half of Area 2 flows toward the northwest, and the south half of Area 2 flows toward the east and northeast. Wells MW-1 and MW-37A are upgradient of Area 2 on the north side of groundwater divide. MW-1 acts as an upgradient well for the southeast side of Area 2. Area 2 is monitored by downgradient water table wells MW-2, MW-2R, MWII-2, MW-4, MW-4B, MW-3, MW-33A, MW-34A, MW-41A and MW-45A, and by bedrock well MW-18. Travel from Area 2 to the east would be intercepted by the leachate toe drain on east side of Area 3. Travel times from Area 2 towards the northwest drainageway are approximately 100-years.

3.3.3 Theoretical Release: Area 3

Figure 35 presents the results of the theoretical release from closed landfill Area 3. Particles were placed within the footprint of Area 3 and at upgradient well locations corresponding to MW-1, MW-32A, MW-37A, and MW-19R. The results of the particle path line modeling show the groundwater flow paths from Area 3. The flow paths show that there is a groundwater divide where the north half of Area 3 flows toward the northwest and the south half of Area 3 flows toward the east and northeast. MW-32A acts as an upgradient well for the southeast side of Area 3. Area 3 is monitored by downgradient water table wells MWII-2, , MW-4, MW-4B, , MW-7A, MW-45A, MW-46A, and by bedrock well MW-22. Travel from Area 3 to the east would be intercepted by the leachate toe drain on north side of Area 1 and east side of Area 3. Travel times from Area 3 towards the northeast toe drain are approximately 100-years.

3.3.4 Theoretical Release: Cell 4

Figure 36 presents the results of the theoretical release from closed landfill Cell 4. Particles were placed within the footprint of Cell 4 and at upgradient well locations corresponding to MW-1, MW-37A and MW-19R. The results of the particle path line modeling show the groundwater flow paths from Cell 4. The flow paths show that groundwater for Cell 4 flows toward the east. MW-32A acts as an upgradient well for Cell 4. Cell 4 is monitored by downgradient water table wells MW-8A, MW-12A, MW-26A, MW-27A, MW-31A, MW-38A, MW-39A, MW-40A and MW-42A, and by bedrock wells MW-11, MW-35, and MW-36. Travel times from Cell 4 towards the tributary of Trout River are approximately 100-years.

3.3.5 Theoretical Release: Cell 5

Figure 37 presents the results of the theoretical release from landfill Cell 5. Particles were placed within the footprint of Cell 5 and at upgradient well locations corresponding to MW-1, MW-37A and MW-19R. The results of the particle path line modeling show the groundwater flow paths from Cell 5. The flow paths show that groundwater for Cell 5 flows toward the east. There are no direct upgradient wells for Cell 5. Cell 5 is monitored by downgradient water table wells MW-7A, MW-24A, MW-25A, MW-29A, MW-31A, MW-42A,

MW-43A, MW-44A, MW-46A, MW-100, and MW-101, and by bedrock wells MW-11, MW-22, and MW-30. Travel times from Cell 5 towards the tributary of Trout River are approximately 200-years.

3.3.6 Theoretical Release: Cell 5 EXP

Figure 38 presents the results of the theoretical release from landfill C 5 EXP. Particles were placed within the footprint of C 5 EXP and at upgradient well locations corresponding to MW-1, MW-37A and MW-19R. The results of the particle path line modeling show the groundwater flow paths from C 5 EXP. The flow paths show that groundwater for C 5 EXP flows toward the northeast and east. There are no direct upgradient wells for C 5 EXP. C 5 EXP is monitored by downgradient water table wells MWII-2, MW-7A, MW-24A, MW-25A, MW-29A, MW-31A, MW-42A, MW-43A, MW-44A, MW-45A, MW-46A, MW-100, and MW-101, and by bedrock wells MW-11, MW-22, and MW-30. Travel times from C 5 EXP towards the tributary of Trout River are approximately 700-years.

3.4 References

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4 Monitoring Points

4.1 Geologic Setting

The Winneshiek County Landfill lies within the Paleozoic Plateau region of northeastern Iowa. Deep valleys, numerous rock outcroppings, high bluffs, and an angular step relief characterize the Paleozoic Plateau's terrain where Quaternary sediments mantle bedrock, and bedrock is the primary control on topography (Prior, 1991).

The landfill is located on a generally eastward sloping upland that is part of an interfluve bordered on the east by an unnamed, northward flowing tributary to the Trout River and on the northwest by an unnamed, northward flowing drainageway that drains to the Trout River (Figure 1).

The geology of site wells is included with the boring logs and monitoring well/piezometer documentation forms (Iowa DNR Form 542-1277) presented in Appendix A. In the Winneshiek County Landfill area, Quaternary-age deposits mantle Ordovician-age bedrock, and the geologic sequence of the Quaternary deposits differs between uplands and valleys. In the uplands, the sequence includes Wisconsin Episode loess overlying either patchy remnants of pre-Illinoian age glacial diamicton ("till") which locally contains discontinuous sand bodies, Quaternary colluvium developed on bedrock, or Ordovician-age bedrock. In the valleys, the sequence consists of alluvium overlying Ordovician-age bedrock. The Ordovician-age bedrock varies across the landfill site. Maquoketa Formation claystone and shale is the uppermost bedrock underlying most of the site, but in the northeastern corner of the landfill area, the Maquoketa Formation is absent because of erosion, and the older Ordovician-age Galena Group Dubuque Formation dolomite is the uppermost bedrock.

4.2 Groundwater Occurrence and Movement

Groundwater is monitored in an interconnected water-bearing zone at the Winneshiek County Landfill: (1) the shallow groundwater measured by the water table, and (2) the deep groundwater measured by the potentiometric surface in the Ordovician-age upper and lower Elgin Member of the Maquoketa Formation and, where the Maquoketa Formation is absent, the Ordovician-age Galena Group Dubuque Formation. Shallow wells are screened in different geologic units because the water table occurs in successively different stratigraphic horizons downslope.

4.2.1 Water Table Occurrence and Groundwater Movement

The water table occurs in successively different stratigraphic units downslope at the Winneshiek County Landfill, including Quaternary-age loess, undifferentiated Pre-Illinoian glacial diamicton ("till"), colluvium, alluvium, Ordovician-age Maquoketa Formation and Galena Group.

Water table elevations are measured in 31 water table wells (Table 3) at the Winneshiek County Landfill. Figure 21 shows the water table elevations for September 2023, which is representative for site conditions. In the immediate landfill area, groundwater generally flows east, northeast, and north from a high near the southwest corner of the landfill area toward the tributaries to the Trout River. Water table wells include:

Upgradient water table wells near the southwest corner of the landfill:

- MW-37A completed in Quaternary-age loess and the underlying pre-Illinoian-age glacial diamicton; this well provides groundwater representative of upgradient water table conditions. This well provides upgradient background only for the west half of Area 1 and Area 2. The well does not provide upgradient background for the entire landfill.
- MW-1 completed in pre-Illinoian-age glacial diamicton and interbedded fluvioglacial material; at this location, landfill activities have affected groundwater quality in MW-1. This well provides upgradient background only for the west half of Area 1 and Area 2. The well does not provide upgradient background for the entire landfill.

 MW-32A completed in pre-Illinoian-age glacial diamicton and the underlying Elgin Member of the Ordovician-age Maquoketa Formation

Water table wells downgradient of Areas 1, 2, and 3 include:

- MW-2 completed in pre-Illinoian-age glacial diamicton and the underlying upper Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-2R completed in the upper Elgin Member of the Ordovician-age Maquoketa Formation bedrock.
- MW-3 completed in pre-Illinoian-age glacial diamicton.
- MW-4 and MW-4B completed in pre-Illinoian-age glacial diamicton and interbedded fluvioglacial material.
- MW-45A completed in the Quaternary Wisconsinan Episode Peoria Loess.
- MW-41A completed in pre-Illinoian-age glacial diamicton and the underlying Ordovician-age upper Elgin Member of the Maquoketa Formation.
- MW-33A completed in pre-Illinoian-age glacial diamicton and the underlying Ordovician-age upper Elgin Member of the Maquoketa Formation.
- MW-34A completed in pre-Illinoian-age glacial diamicton and the underlying Ordovician-age upper Elgin Member of the Maquoketa Formation.

Water table wells downgradient of Cells 4 and 5 EXP include:

- MWII-2 completed in Quaternary Wisconsinan Episode Peoria Loess..
- MW-7A completed in pre-Illinoian-age glacial diamicton. MW-24A and MW-25A completed in fine-grained construction fill overlying thin loess and the underlying Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-29A completed in pre-Illinoian-age glacial diamicton and the underlying Galena Group Dubuque Formation.
- MW-31A completed in pre-Illinoian-age glacial diamicton and the underlying Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-38A completed in Quaternary-age loess and colluvium, the underlying Elgin Member of the Ordovician-age Maquoketa Formation, and the Ordovician-age Galena Group Dubuque Formation.
- MW-39A completed in Quaternary-age loess and colluvium, and the underlying Elgin Member of the Ordovician-age Galena Group Dubuque Formation.
- MW-40A completed in Quaternary-age loess and colluvium, the underlying Ordovician-age Maquoketa Formation.
- MW-42A completed in Quaternary-age loess and colluvium, the underlying Ordovician-age Maquoketa Formation.
- MW-43A completed in Quaternary-age loess and colluvium, and the underlying Elgin Member of the Ordovician-age Galena Group Dubuque Formation
- MW-44A completed in Quaternary-age loess and colluvium, and the underlying Elgin Member of the Ordovician-age Galena Group Dubuque Formation
- MW-45A completed in Quaternary-age loess and colluvium
- MW-46A completed in Quaternary-age loess and colluvium, and the underlying Elgin Member of the Ordovician-age Galena Group Dubuque Formation

Water table wells downgradient of the leachate lagoon include:

 MW-100 and MW-101 completed in Quaternary-age loess and colluvium, and the underlying Elgin Member of the Ordovician-age Galena Group Dubuque Formation.

 MW-43A and MW-44A completed in Quaternary-age loess and colluvium, and the underlying Elgin Member of the Ordovician-age Galena Group Dubuque Formation.

4.2.2 Groundwater Occurrence in the Uppermost Bedrock

Piezometric elevations are measured in the uppermost Ordovician-age bedrock at the Winneshiek Landfill, which includes the upper and lower Elgin Member of the Maquoketa Formation and, where the Maquoketa Formation is absent because of geologic erosion, the geologically older Galena Group Dubuque Formation. Bedrock groundwater elevations are measured in 8 bedrock wells at the Winneshiek County Landfill (Table 3). Figure 22 shows the bedrock groundwater elevations for September 2023, which is representative for site conditions.

Eight wells are completed in the uppermost Ordovician-age bedrock at the Winneshiek County Landfill:

Two upgradient bedrock wells located west of the landfill:

- MW-18 completed in the lower Elgin Member of the Maquoketa Formation.
- MW-19R completed in the lower Elgin Member of the Maquoketa Formation.

Bedrock wells located down gradient of the landfill include:

- MW-11 completed in upper and lower Elgin Member of the Maquoketa Formation.
- MW-22 completed in the Galena Group Dubuque Formation.
- MW-30 completed in the Galena Group Dubuque Formation.
- MW-35 completed in upper and lower Elgin Member of the Maquoketa Formation.
- MW-36 completed in the Galena Group Dubuque Formation.

4.3 Water Table Monitoring Points

The water-table detection monitoring program for the Winneshiek County Landfill, Table 3, includes the monitoring wells shown on Figure 21.

4.3.1 Detection Monitoring

Detection monitoring of water table conditions at Winneshiek County Landfill is comprised of 23 water table wells, and includes:

- Upgradient water table wells MW-1 and MW-37A completed in loess and the underlying pre-Illinoian-age glacial diamicton.
- MW-2R, downgradient of Area 2, completed in the upper Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-3, downgradient of Area 1, completed in pre-Illinoian-age glacial diamicton.
- MW-4 and MW-4B, downgradient of Area 1, completed in pre-Illinoian-age glacial diamicton and interbedded fluvioglacial material.
- MW-7A, downgradient of Cell 5 EXP, completed in pre-Illinoian-age glacial diamicton.
- MW-24A, downgradient of Cell 5 EXP, completed in fine-grained construction fill overlying thin loess and the underlying Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-25A, downgradient of Cells 4 and 5 EXP, completed in fine-grained construction fill overlying thin loess and the underlying Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-29A, downgradient of Cell 5 EXP and leachate lagoon, completed in fine-grained construction fill overlying thin loess and the underlying Galena Group Dubuque Formation.
- MW-31A, downgradient of Cell 5 EXP, completed in overlying thin loess and the underlying Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-33A, downgradient of Areas 2 and 3, completed in fine-grained construction fill overlying thin loess, diamicton and the underlying Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-34A, downgradient of Areas 2 and 3, completed in fine-grained construction fill overlying thin loess, diamicton and the underlying Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-38A downgradient of Cell 4, completed in Quaternary-age loess and colluvium, the underlying Elgin Member of the Ordovician-age Maquoketa Formation, and the Ordovician-age Galena Group Dubuque Formation.
- MW-39A downgradient of Cell 4, completed in Quaternary-age loess and colluvium, and the underlying Elgin Member of the Ordovician-age Galena Group Dubuque Formation.
- MW-40A downgradient of Cell 4, completed in Quaternary-age loess and colluvium, the underlying Ordovician-age Maquoketa Formation.
- MW-41A completed in pre-Illinoian-age glacial diamicton and the underlying Ordovician-age upper Elgin Member of the Maquoketa Formation.
- MW-42A downgradient of Cell 5 EXP, completed in overlying thin loess and the underlying Elgin Member of the Ordovician-age Maquoketa Formation.
- MW-43A downgradient of the leachate lagoon completed in fine-grained construction fill overlying thin loess and the underlying Galena Group Dubuque Formation.
- MW-44A downgradient of the leachate lagoon completed in fine-grained construction fill overlying thin loess and the underlying Galena Group Dubuque Formation.
- MW-45A downgradient of Cell 5 EXP, completed in overlying thin loess
- MW-46A downgradient of Cell 5 EXP, completed in overlying thin loess and the underlying Galena Group Dubuque Formation
- MW-101 downgradient of the leachate lagoon completed in fine-grained construction fill overlying thin loess and the underlying Galena Group Dubuque Formation.

4.3.2 Water Table Assessment Monitoring

There are ten monitoring wells in assessment monitoring. Monitoring wells MW-7A, MW-24A, MW-29A, MW-31A, MW-33A, MW-40A, MW-42A, MA-43A, MW-44A, and MW-46A are all shallow water table monitoring wells. Monitoring wells MW-29A, MW-31A, MW-42A, and MW-43A are currently being evaluated as part of an assessment of corrective measures (ACM) based on Appendix I inorganic metal SSLs (cobalt).

4.3.3 Leachate Manhole Monitoring

There are two leachate manholes, MH-1AA and MH-4-2, which are being monitored as part of an assessment of corrective measures (ACM). These two manholes are being sampled for Appendix I parameters.

4.3.4 Water-Level Measurement Only

Water-level measurement only is performed in the following water table wells:

- MWII-2, downgradient of Area 1 and sidegradient of Area 5, completed in the pre-Illinoian age glacial diamicton.
- MW-2, downgradient of Area 2, completed in the upper Elgin Member of the Ordovician-age Maquoketa Formation. Groundwater levels are too low (<0.1 ft) to sample the well.
- MW-12A, downgradient of Cell 4, completed in fine-grained construction fill and the underlying Elgin Member of the Ordovician-age Maquoketa Formation. Groundwater levels are too low (<0.1 ft) to sample the well.
- MW-26A, downgradient of Cell 4, completed in fine-grained construction fill overlying thin loess and the underlying Elgin Member of the Ordovician-age Maquoketa Formation. Groundwater levels are too low (<0.1 ft) to sample the well.
- MW-27A, downgradient of Cell 4, completed in fine-grained construction fill overlying thin loess and the underlying Elgin Member of the Ordovician-age Maquoketa Formation. Groundwater levels are too low (<0.1 ft) to sample the well.
- MW-32A, sidegradient of Cell 4, completed in loess and the underlying Elgin Member of the Ordovician-age Maquoketa Formation. Groundwater levels are too low (<0.1 ft) to sample the well.
- MW-100 downgradient of the leachate lagoon completed in fine-grained construction fill overlying thin loess and the underlying Galena Group Dubuque Formation. Groundwater levels are too low (<0.1 ft) to sample the well.

4.4 Bedrock Monitoring Points

As shown in Table 3, groundwater quality in the uppermost Ordovician-age bedrock is monitored in four detection monitoring wells and four water-level only measurement wells. The location of these monitoring points is shown on Figure 22.

4.4.1 Detection Monitoring

The four detection monitoring wells in bedrock include:

- Upgradient monitoring well MW-19R completed in the lower Elgin Member of the Maquoketa Formation.
- Upgradient monitoring well MW-18 completed in upper Elgin Member of the Maquoketa Formation
- Downgradient monitoring well MW-11 completed in the upper and lower Elgin Member of the Maquoketa Formation.
- Downgradient monitoring well MW-22 completed in the Galena Group Dubuque Formation.
- Downgradient monitoring well MW-35 completed in upper and lower Elgin Member of the Maquoketa Formation.

4.4.2 Water-Level Measurement Only

The five water-level measurement only wells in bedrock include:

- Downgradient monitoring well MW-30 completed in the Galena Group Dubuque Formation. Groundwater levels are too low (<0.1 ft) to sample the well.
- Downgradient monitoring well MW-36 completed in the Galena Group Dubuque Formation. Groundwater levels are too low (<0.1 ft) to sample the well.

4.4.3 Bedrock Assessment Monitoring

Well MW-19R has replaced upgradient bedrock monitoring well MW-19. Monitoring well MW-19 was in assessment monitoring due to detections for inorganic parameters of arsenic, barium, and nickel, and organic detections for benzene and chlorobenzene. Fall 2023 results and groundwater levels for MW-19R confirm that there were no detections for benzene and chlorobenzene. No parameters were above a GPS.

4.5 Surface Water Monitoring Points

Three surface water locations are monitored (Figure 2). SW-1 is upstream and located on the east by an unnamed, northward flowing tributary to the Trout River. SW-2 is located downstream at the confluence of an unnamed, northward flowing tributary to the Trout River and the Trout River. SW-5 is downstream of SW-1 located on the east an unnamed, northward flowing tributary to the Trout River.

There are two surface water level and flow only monitoring points, SW-3, and SW-4 (Figure 2). Staff gauge SW-3 is downstream of SW-1 and a surface water impoundment on the east side of the landfill. Staff gauge SW-4 is downstream of surface water impoundment on west side of landfill, near the landfill gate.

4.6 Landfill Cell Groundwater Underdrain Monitoring Points

Two landfill cell groundwater drainage layers are monitored (Figure 2). GU-1 is a groundwater drainage layer monitoring location which monitors a section of original landfill Cell 5. GU-2 is a groundwater drainage layer monitoring location which monitors a section of landfill C5 EXP.

4.7 Leachate Head Monitoring Points

Ten landfill leachate head piezometers are monitored (Figure 2). The locations monitor leachate levels for Areas 1, 2, and 3, Cell 4 and Cell 5 EXP.

4.8 Reference

Prior, J.C., 1991. Landforms of Iowa: University of Iowa Press, Iowa City, 154 p.

5 Semi-Annual Groundwater Level Measurement

Groundwater levels will be measured semi-annually at the Winneshiek County Landfill wells in compliance with Iowa Department of Natural Resources Permit No. 96-SDP-1-74P. In addition, leachate head, groundwater underdrain, and surface water elevations are measured. Table 4 lists the monitoring points that will be measured.

For each semi-annual sampling event, field Water Elevation forms, such as the example in Appendix B, will be used to document:

- Well Number
- Elevation of the Top of the Well Casing
- Measured Depth to Water from the Top of Casing
- Measured Depth to the Bottom of the Well

Two sets of groundwater level measurements will be collected from each well prior to groundwater sampling. The first water level measurement will be collected as part of a full round, in which groundwater levels will be collected from all permitted wells within a 24-hour time period for use in evaluating groundwater elevations and flow across the site. A second groundwater level measurement will be collected from each well directly before groundwater sampling.

In preparation for collecting groundwater level and depth to bottom measurements, the groundwater sampling technician shall review the site-specific planning documents to obtain the following information:

- Identification number(s) of the well or wells to be monitored (found in sampling protocols presented in the HMSP);
- Locations of the wells as shown on a site map;
- Records listing the most recent groundwater level measurements and constructed depth to bottoms for the well(s); and
- Reference point information (e.g., top-of-casing elevation, state plane coordinates, marked location of measurement reference point).

Prior to collecting groundwater level measurements, the sampler will visually inspect the measuring tape/probe to ensure that it is not missing sections and the numbers are accurate. The sampler will then decontaminate the water-level indicator in its entirety, including the recessed chamber the probe sits in. Once decontaminated, the probe of the electric water level indicator will be lowered into the riser casing until water is encountered, as indicated by the instrument signal. The instrument signals encountering top of water by illuminating a light on the side of the instrument and by sounding with a continuous beep. The water level will then be measured to the nearest 0.01 foot with respect to the marked "top-of-casing" reference point and entered on the field log. Measurement reference points have been clearly marked on the north side of the top-of-casing for each well. As part of the measurement process, additional consecutive water level measurements will be made by field personnel to verify the initial reading obtained.

The water level measurement will be compared to the most recent water level obtained for the well (previous sampling event). If the measurements differ by more than 0.5 foot, the depth to water will be measured a second time for verification purposes. A remark will be made on the field form if a probable cause for the discrepancy is known (e.g., period of drought, rainfall event, or start-up of a nearby pumping well).

Field measurements of water levels for a given well shall be recorded on the field form including the following information:

- The type of measurement device used;
- Date and time of the measurement; and
- Any pertinent remarks concerning the well condition, instrument malfunction, variation of the measured depth versus the installed depth of the well, soft bottom, etc.

When groundwater-level measurements are taken at the same time as groundwater sampling, the static water level will be taken first; the depth to the bottom of the well will not be taken until after the completion of groundwater sampling to minimize any effects of any sediment on groundwater turbidity and to avoid mixing of groundwater in the well casing prior to groundwater sample collection.

During the groundwater level measurements, other notes will be recorded about the well integrity and recorded on the Water Elevation form. These notes will include information about the status of the well as locked and capped, and any notes about well integrity regarding cracks, obstructions, presence of sediment at bottom of well, presence of insects, or other observed features in the well and surface seal.

Field measurements of surface water levels for a staff gauge shall be recorded on the field form including the following information:

- The type of measurement device used;
- Date and time of the measurement; and
- Any pertinent remarks concerning the staff gauge condition, instrument malfunction, etc.

Annually, photographs of each monitoring location will be taken and a photo log with notes will be completed. The photo log will be part of annual well inspection and submitted as part of the annual monitoring report.

6 Groundwater Sampling and Analysis

This section describes groundwater sampling and analysis at the Winneshiek County Landfill. The sampling and analysis are designed to comply with IAC 567-113.10(455B)(4) and are developed to ensure that monitoring results provide an accurate representation of groundwater quality at the wells at the Landfill.

6.1 Sample Collection and Preservation

Groundwater sample collection will be accomplished using low-flow sampling techniques as described below.

Sample integrity will be maintained by decontaminating field equipment prior to sampling and in between wells, and adhering to the EPA sample collection, preservation, packaging, and chain-of-custody protocols.

6.1.1 Low-Flow Sampling Techniques

For the semi-annual sampling events, low-flow sampling techniques will be used for groundwater sampling. The equipment to be used for this technique will consist of:

- 1. Low-Flow sampling pump and controller (e.g., impeller type pump) (i.e., Proactive Mega-Monsoon) with back-flow check valve, dedicated or non-dedicated bladder pump (i.e., Geotech Portable Stainless-Steel Bladder Pump).
- 2. Disposable tubing made of inert materials for low-flow sampling pump.
- 3. Disposable bladders (if non-dedicated bladder pumps are used).
- 4. Stainless steel bailers or disposable Teflon bailers.
- 5. Electronic water level indicator (two-wire electrical sounder), equipped with a sufficient length of water level tape to reach the deepest anticipated water level; the water level tape should be graduated into 0.01-foot intervals.
- 6. Extra batteries for the water level indicator.
- 7. Disposable nylon rope.
- 8. Liquinox or other non-phosphate detergent.
- 9. Deionized water.
- 10. Pressurized deionized water sprayer.
- 11. Other decontamination equipment, as needed (e.g., brush, plastic bucket, clean spray bottles, paper towels, clean plastic sheeting).
- 12. Five-gallon graduated bucket for collecting and measuring purge volumes.
- 13. Sample containers (provided with appropriate preservatives by Eurofins TestAmerica, Inc.).
- 14. Two coolers for shipment of water samples.
- 15. Ice.
- 16. Myron Ultrameter II or equivalent combination water quality meter (pH, temperature, conductivity).
- 17. LaMotte or equivalent turbidity meter.
- 18. In-Situ Aqua Troll 500 Multiparameter Sonde low-flow system.
- 19. Field data sheets.
- 20. Sample labels.
- 21. Chain-of-custody forms.
- 22. Personal protective equipment (PPE).
- 23. Garbage bags for PPE, filters, and disposable tubing.
- 24. Keys for locked protective casings.
- 25. Tools (e.g., wrenches), as needed, to enter well vault boxes.
- 26. Health and safety monitoring equipment, as needed.

6.1.2 Calibration of Field Equipment

Calibration of all field instruments used to measure and monitor pH, conductivity, turbidity, and temperature will be performed.

The Myron Ultrameter II pH/Conductivity Meter will be calibrated at the beginning of each sampling day. The meter will be calibrated using the manufacturer's specifications. If instrument calibration indicates damage or sensor degradation over time, the instrument will be replaced.

The LaMotte turbidity meter will be calibrated at the beginning of each sampling day. The meter will be calibrated in the field office using the manufacturer's specifications. If instrument calibration indicates damage or sensor degradation over time, the instrument will be replaced.

The In-Situ Aqua Troll 500 probe will be calibrated annually by the manufacturer prior to deployment. The meter will be maintained by the manufacturer using the manufacturer's specifications. A "Quick Cal" which calibrates the basic sensors simultaneously using a single "universal" calibration solution will be performed daily. This will provide the field technician with information on stability of the sensor. If the instrument indicates damage or sensor degradation, the instrument will be replaced.

Prior to sampling, the water level measurement will be compared to the previous day's water level obtained for the well. If the measurements differ by more than 0.5 foot, the depth to water will be measured a second time for verification purposes. A remark will be made on the field form if a probable cause for the discrepancy is known (e.g., rainfall event, drought, or start-up of a nearby pumping well).

Depth to bottom of the well will be measured to the nearest 0.01 foot in a similar manner, but only after groundwater sampling has been completed to minimize the disturbance of any fine-grained sediment which may be present at bottom of well. As part of the measurement process, additional consecutive depth to bottom measurements will be made by field personnel to verify the initial reading obtained. Any discrepancy between the measured well depth and constructed well depth shall be noted as a remark on the form; such a discrepancy may indicate the presence of a possible obstruction or break in the casing or sedimentation at the bottom of the monitoring well.

Prior to groundwater sampling, the well depth to water will be measured. The pump and its tubing will then be slowly lowered into the monitoring well until the pump intake is situated at a predetermined level within the screened interval of the water column. The discharge line from the pump will then be connected to the flow-through cell, and the discharge line from the flow-through cell will be directed to a container to collect wastewater. Pump flow rates will be set to maintain a steady flow with the goal of maintaining a drawdown of no more than 0.33 feet. Flow rates will be measured using a graduated cylinder or beaker and a stopwatch.

The flow-through cell will be used to monitor several parameters directly. One tubing volume of water will be pumped to the waste container before readings will be initiated. Stabilization will be considered to have occurred when the parameters listed in Table 5 have stabilized on three successive readings within the listed criteria:

Table 5 Low-Flow Sampling Stabilization Parameters

Table 6 summarizes the sample containers and preservatives to be used. All samples will be stored in coolers containing ice in a secure area until custody is relinquished.

Bottle and Preservation Requirements

Following sampling, well integrity will be checked by measuring the well depth to bottom. The well depth to bottom and static water level will be compared to previous monitoring events to check for potential changes to well depth to bottom. Any changes to well depth to bottom or well integrity will be noted on field data forms.

6.1.3 Decontamination

The outer parts of the bladder pump that have been in contact with groundwater will be cleaned prior to sampling and in between wells by being rinsed with deionized water. The inner parts of the pump will be cleaned by pumping ½ gallon of deionized water through the pump head. The pump uses a disposable bladder and tubing, which is replaced after each well. Rinsates will be discharged to a 5-gallon water bucket and ultimately disposed of by discharging to the landfill surface.

Personal protective equipment (PPE) that is generated throughout sampling activities shall be placed in plastic garbage bags. All PPE should be disposed as non-hazardous waste in the designated on-site rolloff box at the Landfill. Trash that is generated as part of field activities may be disposed of in the Landfill as long as the trash was not exposed to hazardous media.

Disposable Teflon bailers will not require decontamination. If used, they will be disposed with the PPE in a plastic garbage bag.

PPE will be used during decontamination. PPE will include nitrile gloves, safety glasses, steel-toe boots, and poly-coated Tyvek, if necessary, to protect against dermal contact with groundwater.

6.1.4 Quality Control

To verify the quality of the sampling process, sample blanks and duplicates will be collected. One equipment (rinsate) blank will be collected for every 20 samples or a minimum of once per sampling event. The equipment blank will be collected after the decontamination process (described in Section 6.1.3) by collecting deionized water through the sampling device in the appropriate bottles listed in Table 6.

Field duplicates will be collected for every 10 or fewer samples. When a field duplicate sample is collected, it will be a second sample collected consecutively from a well using the sampling device.

To assess the degree and type of accidental contamination by volatile organics during the sample collection and shipment procedures, two 40-ml volatile trip blank samples per cooler will be filled completely with distilled water by the laboratory prior to mobilization to the site and stored with the sample volatile vials to be used during field sample collection.

Field documentation will undergo an internal QC review after the completion of field activities. Field forms will be reviewed by the AECOM QA Manager for completeness, accuracy, and compliance with this document. Upon completion of the field sampling events, field documentation will be relinquished to the Project Manager and digital copies stored on an AECOM computer server.

6.1.5 Documentation

Data collected and observations made during groundwater sampling will be recorded on the Well Purging and Sample Collection Field Data Sheet (see Appendix C).

6.2 Sample Shipment

Samples will be collected from each monitoring point and submitted by AECOM personnel to Eurofins Environment Testing North Central, LLC (Eurofins – Cedar Falls)., located in Cedar Falls, Iowa at the end of each sampling day or in the morning the day after sampling.

6.3 Analytical Procedures

Measurements of pH, conductivity, turbidity, and temperature will be performed in the field using a pH/conductivity/turbidity/temperature meter. Eurofins – Cedar Falls, will conduct all other analyses. Table 7 provides a list of all field and laboratory (IDNR Appendix I) analyses that will be performed along with method detection limits and reporting limits.

6.4 Chain-of-Custody Control

The following sections summarize the sample handling and chain-of-custody procedures that will be used for this project and include an example Chain-of-Custody (Appendix D) form. The Chain-of-Custody (COC) form is the written documented history of a sample and is initiated at the time of sampling. This form will be completed by AECOM sampling personnel and will accompany the samples to the laboratory where it will be received and stored under the laboratory's custody. The purpose of the COC form is to provide a legal written record of the handling of samples from the time of collection until they are received at the laboratory. It also serves as the primary written request for analyses from AECOM to the laboratory. The COC form also acts as a purchase order for analytical services when no other contractual agreement is in effect.

The information the AECOM sampler will provide at the time of sampling on the container label is:

- Sample identification
- Date and time
- **Preservative**

During the sampling process, the COC form will be completed and must be legible. This form will include the following information:

- Client name, address, phone number, and fax number (if available).
- Project Name and/or Number.
- The sample identification.
- Date, time, and location of sampling.
- Sampling plan, if applicable.
- Sample collector's name.
- The matrix description.
- The container description.
- The total number of each type of container.
- Preservatives used.
- Analysis requested.
- Requested turnaround time (TAT).
- Any special instructions.
- Purchase order number or billing information (e.g., quote number), if available.
- The date and time each person received or relinquished the sample(s), including their signed name.

Samples will remain solely in the possession of the AECOM field technician until delivery to laboratory personnel. Samples are stored in a cooler with ice, as applicable. The sample collector will assure that each container is always in his/her physical possession/view or stored in such a place and manner to preclude tampering. The field technician will relinquish the samples in writing on the COC form to laboratory sample control personnel. Samples are only considered to be received by the lab when personnel at the laboratory have physical contact with the samples.

Samples delivered to the laboratory must meet the following criteria:

- 1) Samples must be properly labeled.
	- Use durable labels (labels provided by Eurofins Cedar Falls are preferred).
	- Include a unique identification number.
	- Include sampling date and time and sampler ID.
	- Include preservative used.
	- Use indelible ink.
- 2) Proper sample containers with adequate volume for the analysis and necessary QC are required for each analysis requested.
- 3) Samples must be preserved according to the requirements of the requested analytical method.
- 4) Most analytical methods require chilling samples to 4°C (exceptions would include samples submitted for metals analysis). For methods requiring temperature preservation to 4°C, the criteria are met if the samples are chilled to at or below 6°C and above freezing. For methods with other temperature criteria, the samples must arrive within $\pm 2^{\circ}C$ of the required temperature, or within the method specified range. Note: Samples that are hand-delivered to the laboratory immediately after collection may not have had time to cool sufficiently. In this case, the samples will be considered acceptable as long as there is evidence that the chilling process has begun (arrival on ice).
- 5) All samples submitted for Volatile Organic analyses should have a Trip Blank submitted with the samples. Eurofins – Cedar Falls will supply a Trip Blank, if requested, with all bottle orders containing Volatile Organic analyses.
- 6) The Eurofins Cedar Falls project manager will be notified if any sample is received in damaged condition. Eurofins TestAmerica will then contact AECOM with the details of the sample condition upon receipt and request further instructions.

6.4.1 Sample Receipt

When samples arrive at the laboratory, designated sample-receiving personnel inspect the coolers and samples. The integrity of each sample must be determined by comparing sample labels or tags with the COC and by visual checks of the container for possible damage. Any non-conformance, irregularity, or compromised sample receipt must be documented and brought to the immediate attention of AECOM. The COC; shipping documents; documentation of any non-conformance, irregularity, or compromised sample receipt; record of client contact, and resulting instructions become part of the project record.

6.4.2 Sample Storage

To avoid deterioration, contamination, or damage to a sample during storage and handling, from the time of receipt until all analyses are complete, samples are stored in refrigerators suitable for the sample matrix. In addition, samples to be analyzed for volatile organic parameters are stored in separate refrigerators designated for volatile organic parameters only. Samples are never to be stored with reagents, standards, or materials that may create contamination.

To ensure the integrity of the samples during storage, refrigerator blanks are maintained in the volatile sample refrigerators and analyzed every 2 weeks.

Analysts and technicians retrieve the sample container allocated to their analysis from the designated refrigerator and place them on carts, analyze the sample, and return the remaining sample or empty container to the refrigerator from which it originally came. All unused portions of samples, including empty sample containers, are returned to the secure sample control area. All samples are kept in the refrigerators for 2 to 4 weeks after analysis, which meets or exceeds most sample holding times. Special arrangements may be made to store samples for longer periods of time. This extended holding period allows additional metal analyses to be performed on the archived sample and assists in dealing with legal matters or regulatory issues.

Access to the laboratory is controlled such that sample storage need not be always locked unless a project specifically demands it. Samples are accessible to laboratory personnel only. Visitors to the laboratory are prohibited from entering the refrigerator and laboratory areas unless accompanied by an employee of Eurofins TestAmerica.

6.5 Quality Assurance and Quality Control

6.5.1 Field Quality Control

The field team will be responsible for inspecting sample containers before leaving for the field. Only new sealed sample containers accompanied by the manufacturer's certification of precleaning will be used. The sample containers will also be inspected for cracks, ill-fitting lids, and other obvious defects before use and will be discarded if defects are found to be present.

The pH/conductivity meter will be calibrated at the beginning of each sampling day. The meter will be calibrated using the manufacturer's specifications. The calibration for the pH function will be at least a 2-point calibration (pH 7.0 and pH 4.0 solutions) and will occur in the office (except for the initial calibration check which will be done at the time of checkout). Calibration for the conductivity function will be performed with a potassium chloride (KCl) solution in the office. The meter does have temperature compensation; therefore, temperature differences between the sample and the calibration standards will not be an issue.

During water quality sampling, one equipment blank will be collected for every 20 or fewer groundwater samples when non-dedicated sampling equipment is used. Field duplicate samples will be at a frequency of 1 per 10 groundwater samples. A trip blank will be included in each shipment of samples collected for volatile organic compounds (VOCs).

Field quality control samples will be evaluated during data validation and the appropriate qualifiers assigned. For detections in blanks at concentrations below the reporting limit, the associated sample results that are less than the reporting will be qualified non-detect (U). For detections in the blanks that are greater than the reporting limit, associated sample concentrations within five times the blank concentration will be qualified non-detect.

Field equipment requiring testing, inspection and maintenance includes the pH/conductivity meter. This meter will be used to measure pH, temperature, and conductivity for water samples while in the field. The manufacturer's operating manual for this instrument describes the procedures for testing and inspecting the meter. These procedures include a battery check, verification that the meter was successfully calibrated

during its previous use and ensuring preventative maintenance has been completed per the manufacturer's recommendations.

An inspection checklist and initial calibration check will be completed by a field team member upon checkout of the meter. Calibration standards and commonly needed spare parts will also be obtained upon checkout. Any preventative or corrective maintenance done will be documented in the equipment log.

Other field equipment includes the bladder pump with back-flow check Valve. Inspection and maintenance of the pump will consist of checking for degradation, replacing the bladder, replacing the tubing, and cleaning the pump. Cleaning will include an Alconox (or Liquinox) wash and rinse, followed by a deionized water rinse of both the exterior and interior.

6.5.2 Laboratory Quality Control

Eurofins – Cedar Falls, procedures include reviewing the instrument log for any notations regarding problems experienced during the previous use and verifying the preventative maintenance has been completed per the manufacturer's recommendations. Any preventative or corrective maintenance done will be documented in the maintenance log. Spare parts are kept in the laboratory's supply room and are available when needed.

Eurofins – Cedar Falls, internal standards address the calibration for the laboratory instruments. Information regarding the standards used will be documented in the instrument log.

Eurofins – Cedar Falls, analyst assigned to conduct the analysis will be responsible for inspecting equipment and supplies upon receipt. The manufacturer's specifications for product performance and purity will be used as the acceptance criteria.

6.6 References

- Puls, R.W., and M.J. Barcelona. 1996. Low Flow (Minimal Drawdown) Ground Water Sampling Procedures. USEPA/ORD EPA/540/S-95/504. Washington, DC: US Environmental Protection Agency.
- Wilde, F.D., D.B. Radtke, J. Gibs and R.T. Iwatsubo. Eds., 1998. National Field Manual for the Collection of Water-Quality Data; U.S. Geological Survey Techniques of Water-Resources Investigations, Book 9, Handbooks for Water-Resources Investigations, Variously Paginated.
7 Detection Monitoring Program

The detection monitoring program for the Winneshiek County Landfill has been established in accordance with the IDNR regulations in 567-113.10(5).

7.1 Monitoring Points

The detection monitoring system consists of 27 monitoring wells, including 23 shallow (water table) wells:

MW-1 MW-2R MW-3 MW-4 MW-4B MW-7A MW-24A MW-25A MW-29A MW-31A MW-33A MW-34A MW-37A MW-38A MW-39A MW-40A MW-41A MW-42A MW-43A MW-44A MW-45A MW-46A MW-101

It also includes four deeper wells in the uppermost bedrock:

MW-11 MW-19R MW-18 MW-22 MW-35

In addition to the monitoring wells, underdrain outlets (GU-1 and GU-2) are also monitored as part of the water table monitoring system. Surface water is monitored by upgradient surface water monitoring point SW-1 and downgradient surface water monitoring points SW-2 and SW-5, located just east of the landfill on an unnamed tributary to the Trout River. Leachate manholes, MH-1AA and MH-4-2 are also monitored as part of the ACM.

A summary of the hydrologic monitoring system is provided in Table 3. Locations of detection monitoring points are shown in Figure 2.

7.2 Comparison of Monitoring Points

7.2.1 Shallow (Water Table) Monitoring Points

Background Points

Based on the site hydrogeology, the Landfill overlies a groundwater divide in the water table (Figure 21). On the west side of the divide, water table flow is toward the west-northwest. MW-1 and MW-37A are hydraulically upgradient of Landfill Areas 1 and 2; and these wells are screened in Quaternary loess and the underlying glacial diamicton. However, these wells are not hydraulically upgradient of the monitoring wells located on the eastern side of the groundwater divide.

In the past, MW-1 has been used as an upgradient background well. In October 2009, a confirmed detection of chlorobenzene, below the 100 µg/L GPS, occurred at this location, and the well was placed in assessment monitoring. Since November 2014, MW-1 has not had any chlorobenzene or other organic detections. MW-1 is not currently in assessment monitoring and has returned to semi-annual Appendix I sampling.

Well MW-32A is hydraulically upgradient of Landfill Area 3, and Cells 4 and 5EXP; and is screened in Quaternary loess and the underlying glacial diamicton. However, this well has insufficient groundwater for sampling.

Given that, wells MW-1 and MW-37A will be used as upgradient background for all water table wells.

Downgradient

The downgradient water table wells at the Landfill are generally screened in different hydrostratigraphic units than those of the upgradient wells (Table 3). The groundwater geochemistry also naturally varies between the different hydrostratigraphic units as discussed in Section 2.15. Interwell (upgradient vs. downgradient) comparisons of groundwater quality is the statistical comparison method for the downgradient water table at the Landfill.

Historical groundwater sampling data has shown two water table wells which have displayed impacts due to landfill activity: MW-4, and MW-4B. MW-4 and MW-4B were installed due to organic parameters detected in groundwater. A leachate toe drain was installed to intercept leachate from Areas 1, 2, and 3 and mitigate interaction of leachate with groundwater from these unlined cells. The remedy has been effective in reducing and eliminating organics detected in groundwater as shown in historical sample results for MW-4 and MW-4B. As a result, MW-4 and MW-4B have returned to semi-annual Appendix I sampling.

7.2.2 Deep (Uppermost Bedrock) Monitoring Points

Upgradient

Based on the site hydrogeology, the Landfill overlies groundwater occurring deep in a bedrock aquitard (Figure 21). Upgradient wells MW-18 and MW-19R are the only upgradient deep detection wells at the Landfill; both wells are screened in the Ordovician-age lower Elgin Member of the Maquoketa Formation shale. The groundwater geochemistry also naturally varies between the different hydrostratigraphic units as discussed in Section 2.15. Based on bedrock groundwater flow, bedrock wells MW-18 and MW-19R are used for upgradient background. Interwell well comparisons of current groundwater quality to previous years' background data is the statistical comparison method for bedrock groundwater at the Landfill.

Downgradient

Deep downgradient well MW-11 and MW-35 are screened in the Ordovician-age lower Elgin Member of the Maquoketa Formation shale. Deep downgradient well MW-22, is screened in the Galena Group Dubuque Formation dolomite. Wells MW-30 and MW-36 have insufficient groundwater for sampling. Two of the five downgradient bedrock wells are screened in a different hydrostratigraphic unit than that of the upgradient well (Table 3). Regardless, interwell comparisons of groundwater quality is the statistical comparison method for the downgradient bedrock at the Landfill.

7.2.3 Surface Water Monitoring Points

Monitoring point SW-1 is upgradient (upstream) of the Landfill, and interwell comparisons of current surface water quality to previous years' background data is the statistical comparison method used for this location.

Monitoring points SW-2 and SW-5 are downgradient (downstream) from the Landfill and is compared statistically to the upstream monitoring point SW-1.

7.3 Statistical Analysis

Statistical analysis for the detection monitoring program for the Winneshiek County Landfill has been established in accordance with the IDNR regulations in 567-113.10(4).

Groundwater samples from the detection monitoring points will be sampled and analyzed semi-annually, weather dependent, in Spring (March/April/May) and Fall (September/October/November) for IDNR Appendix I parameters. Monitoring data and statistical evaluations will be submitted to IDNR within the required 14 days (Subrules 113.10(5) and 113.10(6)) as they are completed.

Prior to statistical analysis, the data will be examined to observe results that may be artificially high or low (outliers) due to potential field sampling, transportation, laboratory, or transcription errors.

Low flow sampling began in the fall of 2014. Therefore, interwell background levels of parameters detected in the current sampling year will be established by pooling all available historical data collected since Fall of 2014 from the appropriate background data set through the end of the preceding year (i.e., 2014 – 2023 for AWQR 2024). Measurements of turbidity and total suspended solids (TSS) which can affect measurement of inorganic metals parameters also began in Fall of 2014. For the detection monitoring program, current-year compliance data will be statistically tested against the background data using prediction limit methods that utilize the comparisons described in Section 7.2. The procedure used to determine the appropriate prediction limit method will be as follows:

- 1. If a monitoring well's background for a given parameter contains more than 50 percent non-detects, a nonparametric prediction limit will be used to test the interwell compliance data against interwell background, and the reporting limit (RL) will be used for nondetect values to account for non-detects.
- 2. If a monitoring well's background for a given parameter contains between 20 percent and 50 percent non-detects, Cohen's adjustment will be used to account for nondetect values.
- 3. If a monitoring well's background for a given parameter contains less than 20 percent non-detects, the RL will be used to account for nondetect values.
- 4. If monitoring well's background for a given parameter contains less than 50 percent non-detects, the background data will be tested for lognormality and normality using the Shapiro-Wilk procedure and probability plots. If background for a given parameter is consistent with a lognormal or normal distribution (defined as a nonsignificant result for the Shapiro-Wilk method at a 95 percent confidence level) and contains less than 50 percent non-detects, a parametric prediction limit (USEPA Unified Guidance 99 percent Confidence One-Sided) will be used to test the interwell compliance data against interwell background.
- 5. If a monitoring well's background data for a given parameter is not found to be either lognormal or normal in statistical distribution (defined as a significant result for the Shapiro-Wilk procedure at a 95 percent confidence level), a distribution-free (nonparametric) prediction limit (99 percent confidence level) will be used to test the interwell compliance data against interwell background.

A pass 1-of-2 verification resampling scheme will be utilized for sampling locations which statistically exceed (SSI) the interwell background prediction limit and for wells which exceed a GWPS. For current year monitoring well compliance data, an exceedance of the appropriate interwell prediction limit for a given Appendix I inorganic constituent will trigger a retest for that constituent as part of a verification resampling event. If the retest result also exceeds the prediction limit, a statistically significant increase (SSI) over background levels at that well will be considered to have occurred. If it cannot be demonstrated that a source other than a landfill unit caused the change in concentration or that the SSI resulted from error in sampling, analysis, statistical evaluation, or natural variation in groundwater quality, the well will enter assessment monitoring.

Because Appendix I organic constituents are anthropogenic, detection of a given organic constituent at a single detection monitoring well will trigger a retest for that constituent as part of a verification resampling event. If the retest result also indicates a detection, a confirmed SSI over background levels will be considered to have occurred. If it cannot be demonstrated that a source other than a landfill unit caused the change in concentration or that the SSI resulted from error in sampling, analysis, statistical evaluation or natural variation in groundwater quality, the well will enter assessment monitoring.

7.4 Site-specific Groundwater Protection Standards (GWPS)

This purpose of this section is to present the results of estimating a site-specific groundwater protection standard for cobalt. Background data for cobalt is based on upgradient interwell shallow groundwater monitoring wells. Wells included are: MW-1, MW-1R, MW-19R, and MW-37A. Except for MW-19R, sample dates span from May 2014, when low-flow sampling began, through June 2024.

Sampling from these non-impacted monitoring points has indicated the measured background concentrations of total cobalt are occasionally above the GWPS in wells screened in the groundwater monitoring zone. Previously described site conceptual groundwater flow is recharge from precipitation moves vertically downward into glacial aged sediments consisting of loess, glacial tills, colluvium, and recharges bedrock composed of Maquoketa shale and claystone and Galena Group limestone. The Maquoketa bedrock has been eroded as part of last glaciation from west to east across the site as part of development of the Trout River to where the top of bedrock on the east side consists of Galena Group limestone. The Galena Group limestone is a uniform microcrystalline limestone with few fractures. Hence groundwater flows along the Maquoketa Group claystone and shale contact with Galena discharging as springs to tributary of Trout River. As such, groundwater chemistry is a mix of upgradient water table and bedrock groundwater flowing and mixing to east which then discharging to the Trout River.

Given the above, development of a site-specific background GWPS for cobalt is suitable. The paragraphs below provide a detailed discussion and justification for a background GWPS for cobalt.

The process for estimating GWPS for annual sample results comparisons includes analysis for trends, outliers, goodness-of-fit (GOF), and prediction limits (PL). Currently, background for the facility includes data from May 2014 through Spring 2024.

First, detection monitoring data for each monitoring well used in site-specific GWPS were tallied for sample size (n), count of nondetects and percentage of nondetects per well for cobalt. For wells with 90- 100% nondetect per parameter, no outliers, trends or goodness of fit.

Next, detection monitoring data, both raw and log transformed, for each monitoring well used in sitespecific GWPS were analyzed for outliers using Rosner's or Dixon's methods depending on sample size. If there are significant outliers in the detection monitoring data which may be attributed to errors in field sampling, lab methods, changes in lab detection limits, or lab data recording, identified errors, which can be resolved, would be corrected and outliers removed. If not, the outlier, would remain in the data set.

Next, if there were no significant outliers, the monitoring well detection monitoring data was analyzed for trends using the Helsel (2012) NADA Akritas-Theil-Sen slope and Kendall's tau methods. If there were no significant trends within the past 10 years of detection monitoring data, the background data set was pooled to estimate the site-specific cobalt GWPS.

Next, if there were no significant outliers, the pooled background chemistry data would be analyzed for statistical distribution using goodness-of-fit tests. Data will be tested for normal, lognormal and gamma distributions. If the data did not follow one of those distributions, nonparametric methods were used to estimate the nonparametric prediction limits.

Based on goodness-of-fit (distribution), nonparametric prediction limits were estimated. The site-wide prediction limit for cobalt in groundwater is estimated to be 3.40 µg/L and is being used in lieu of IDNR GWPS of 2.1 μ g/L for all groundwater comparisons.

8 Assessment Monitoring Program

Assessment monitoring at the Winneshiek County Landfill will be completed in accordance with IDNR regulations in 567-113.10(6).

8.1 Sampling Program

Within 90 days of being triggered into assessment monitoring, as described in Section 7 of this document, groundwater from the assessment monitoring point will be analyzed for IDNR Appendix II constituents. For any Appendix II specific constituent detected at or above the reporting limit, in the monitoring point's sample as a result of the complete Appendix II analysis, a minimum of five quarterly samples from the monitoring point will be collected and analyzed to establish background for detected constituents.

Wells that are in assessment monitoring will be analyzed annually for all detected IDNR Appendix II constituents, and five quarterly background samples from wells with detected Appendix II constituents will be obtained to establish background levels for analytes. After obtaining the sampling results, a notice will be placed in the operating record within 14 days identifying the Appendix II constituents that were detected and the IDNR will be notified that the notice has been placed in the operating record. Wells which have no detections of Appendix II only constituents after two Appendix II sampling events will be re-tested for complete Appendix II list once every five years.

8.2 Statistical Analysis

At assessment monitoring points, comparisons of all constituents detected in the current monitoring year will be made to interwell background levels as detailed in Sections 7.2 and 7.3 of this document.

In addition to comparisons to background, comparisons of all constituents detected in the current monitoring year to the appropriate statewide standards for a protected groundwater (GWPS) source will be performed using either a parametric or a nonparametric confidence interval as appropriate at a 99 percent confidence level (α = 0.01). The current statewide groundwater protection standards (GWPS) for a protected groundwater source used in the comparisons will be obtained from the following IDNR web site:

https://programs.iowadnr.http://programs.iowadnr.gov/riskcalc/pages/standards.aspx.

Prior to statistical analysis, the data will be examined to observe results that may be artificially high or low (outliers) due to field, transportation, laboratory, or transcription errors.

Levels of parameters detected in the current sampling year will be established by pooling all available historical data collected since the fall of 2014 (i.e., 2014 – 2023 for AWQR 2024). The procedure used to determine the appropriate prediction limit method will be as follows:

- 1. If the monitoring well's historical data for a given parameter contains more than 50 percent non-detects, a nonparametric confidence interval will be used to test the intrawell compliance data against the GWPS, and the reporting limit (RL) will be used for nondetect values to account for non-detects.
- 2. If the monitoring well's historical data for a given parameter contains between 20 percent and 50 percent non-detects, Cohen's adjustment will be used to account for nondetect values.
- 3. If the monitoring well's historical data for a given parameter contains less than 20 percent non-detects, the RL will be used to account for nondetect values.
- 4. If the monitoring well's historical data for a given parameter contains less than 50 percent non-detects, the historical data will be tested for lognormality and normality using the Shapiro-Wilk procedure and probability plots. If the monitoring well's historical data for a given parameter is consistent with a lognormal or normal distribution (defined as a nonsignificant result for the Shapiro-Wilk method at a

95 percent confidence level) and contains less than 50 percent non-detects, a parametric confidence interval will be used to test the intrawell compliance data against the GWPS.

5. If the monitoring well's historical data for a given parameter are not found to be lognormal or normal in statistical distribution (defined as a significant result for the Shapiro-Wilk procedure at a 95 percent confidence level), a nonparametric confidence interval (99 percent confidence level) will be used to test the intrawell compliance well data against the GWPS.

If the concentrations of all Appendix II constituents are statistically shown to be at or below a monitoring well's background values and below the applicable GWPS for three consecutive sampling events, the IDNR will be notified of this finding and the monitoring point will return to detection monitoring.

If the concentrations of any Appendix II constituents are above a monitoring point's background values, but all concentrations are statistically shown to be below the applicable GWPS, the monitoring point will continue in assessment monitoring.

If one or more Appendix II constituents are detected at statistically significant levels above the applicable GWPS in any sampling event, within 14 days of this finding, a notice will be placed in the operating record identifying the Appendix II constituents that have exceeded the GWPS, and IDNR and all other appropriate local government officials will be notified that the notice has been placed in the operating record.

If one or more Appendix II constituents are detected at statistically significant levels above the applicable GWPS in any sampling event, an attempt will be made to determine whether a source other than a landfill unit caused the change in concentration, or whether the SSI resulted from error in sampling, analysis, statistical evaluation, or natural variation in groundwater quality. If this is found to be the case, a report documenting this demonstration will be certified by a qualified groundwater scientist and submitted to IDNR for approval, and assessment monitoring will continue. If the concentrations of all Appendix II constituents are subsequently shown to be at or below a monitoring point's background values for two consecutive sampling events, the IDNR will be notified of this finding and may return to detection monitoring.

If the monitoring point's detections cannot be demonstrated to be unrelated to Landfill activities, the following actions will be taken within 90 days:

- 1. Characterize the nature and extent of the release by installing additional monitoring wells, as necessary, until the horizontal and vertical dimensions of the groundwater concentrations have been defined to establish the spatial extent of the changed groundwater concentrations;
- 2. Install at least one additional monitoring well at the facility boundary in the direction of changed groundwater concentrations and sample this well to determine if changed concentrations are migrating off site;
- 3. Notify all persons who own the land or reside on the land that directly overlies any part of the changed groundwater concentrations if the spatial extent of the changed concentrations has migrated off-site when indicated by sampling of wells; and
- 4. Initiate an assessment of corrective measures (ACM).

Assessment of corrective measures (ACM) is discussed in Section 9 of this document.

9 Assessment of Corrective Measures

9.1 Assessment of Corrective Measures (ACM)

An assessment of corrective measures will be completed once it has been determined that any of the Appendix I or Appendix II constituents have been detected and confirmed at a statistically significant level that exceeds the groundwater protection standards (SSL). This assessment will be completed in accordance with the IDNR regulations in 567-113.10(7).

An ACM will be initiated within 90 days of finding that any of the constituents listed in Appendix I or Appendix II have been detected at a statistically significant level exceeding the groundwater protection standards (GWPS). Within 180 days of the initial finding, an ACM will be completed and submitted to the IDNR for review and approval unless otherwise authorized or required by the IDNR. Monitoring will continue in accordance with the assessment monitoring program as outlined in Section 6.

The ACM will analyze the effectiveness of potential corrective measures in addressing the following:

- 1. The performance, reliability, ease of implementation, and potential impacts of appropriate potential remedies, including safety impacts, cross-media impacts, and control of exposure to any residual contamination;
- 2. The time required to begin and complete the remedy;
- 3. The costs of remedy implementation; and
- 4. The institutional requirements, such as state or local permit requirements or other environmental or public health requirements, that may substantially affect implementation of the remedy(remedies).

Within 60 days of approval from the IDNR of the ACM, and prior to the selection of a remedy, a public meeting will be held with interested and affected parties to discuss the results of the corrective measures assessment. The IDNR may establish an alternative schedule for completing the public meeting requirement. Notice of the public meeting will be sent to all owners and occupiers of property adjacent to the permitted boundary of the facility, the IDNR and the appropriate IDNR field office. A copy of the minutes of this public meeting and the list of community concerns will be placed in the operating record and submitted to the IDNR.

9.2 Selection of Remedy

Based on the results of the ACM described above, a remedy will be selected in accordance with the IDNR regulations in 567-113.10(8) within 60 days of holding the public meeting that, at a minimum, meets the following standards:

- 1. Be protective of human health and the environment;
- 2. Attain the groundwater protection standards described in Section 8 of this document;
- 3. Control the source(s) of releases to reduce or eliminate, to the maximum extent practicable, further releases of Appendix I or Appendix II constituents into the environment that may pose a threat to human health or the environment; and
- 4. Comply with standards for management of wastes that are protective of human health and the environment, and that comply with applicable RCRA, state and local requirements.

IDNR may establish an alternative schedule for selecting a remedy after holding the public meeting. Within 14 days of selecting a remedy, a report will be submitted to IDNR describing the selected remedy and explaining how the selected remedy meets the standards listed above.

In selecting a remedy, the following evaluation factors will be considered:

- 1. The long-term and short-term effectiveness and protectiveness of the potential remedy(remedies), along with the degree of certainty that the remedy will prove successful.
- 2. The effectiveness of the remedy in controlling the source to reduce further releases.
- 3. The ease or difficulty of implementing a potential remedy(remedies).
- 4. Practicable capability of the owner or operator, including a consideration of technical and economic capabilities.
- 5. The degree to which community concerns, including but not limited to the concerns identified at the public meeting, are addressed by a potential remedy(remedies).

The selected remedy will include a schedule(s) for initiating and completing remedial activities. Such a schedule will require the initiation of remedial activities within a reasonable time period. The following factors will be considered in determining the schedule of remedial activities:

- 1. Extent and nature of contamination;
- 2. Practical capabilities of remedial technologies in achieving compliance with groundwater protection standards, and other objectives of the remedy;
- 3. Availability of treatment or disposal capacity for wastes managed during implementation of the remedy;
- 4. Desirability of utilizing alternative or experimental technologies that are not widely available, but which may offer significant advantages over already available technologies in terms of effectiveness, reliability, safety, or ability to achieve remedial objectives;
- 5. Potential risks to human health and the environment from exposure to contamination prior to completion of the remedy;
- 6. Resource value of the aquifer;
- 7. Practicable capability of the owner or operator; and
- 8. Other relevant factors.

9.3 Implementation of Corrective Action Plan

Once the remedy has been selected, the corrective action plan will be implemented in accordance with the IDNR regulations in 567-113.10(9). Based on the schedule, the owner or operator will:

- 1. Establish and implement a corrective action groundwater monitoring program that meets the requirements of an ACM, indicates the effectiveness of the corrective action remedy, and demonstrates compliance with groundwater protection standards.
- 2. Implement the selected corrective action remedy; and
- 3. Take any interim measures necessary to ensure the protection of human health and the environment. Interim measures will, to the greatest extent practicable, be consistent with the objectives of and contribute to the performance of any remedy that may be required. The following factors will be considered in determining whether interim measures are necessary:
	- Time period required to develop and implement a final remedy;
	- Actual or potential exposure of nearby populations or environmental receptors to hazardous constituents;
	- Actual or potential contamination of drinking water supplies or sensitive ecosystems;
- Further degradation of the groundwater that may occur if remedial action is not initiated expeditiously;
- Weather conditions that may cause hazardous constituents to migrate or be released;
- Risk of fire or explosion, or potential for exposure to hazardous constituents as a result of an accident or the failure of a container or handling system; and
- Other factors that may pose threats to human health and the environment.

In the case, where the selected remedy fails to achieve the goals of remediation, the owner or operator will notify the IDNR and implement other methods or techniques that could satisfy the goals, unless it is determined that achieving the goals cannot be practicably achieved with any currently available methods. The notification to IDNR will either explain how the proposed alternative methods or techniques will meet the goals, or else it will indicate that the goals cannot be practicably achieved with any currently available methods. Within 90 days of approval by the IDNR for the proposed alternative methods or techniques or the determination of impracticability, the owner or operator will implement the proposed alternative methods or techniques or implement alternative measures.

In the case of a determination of impracticability, the owner or operator will:

- 1. Obtain certification of a qualified groundwater scientist and approval by the IDNR that compliance cannot be practicably achieved with any currently available methods;
- 2. Implement alternate measures to control exposure of humans or the environment to residual contamination, as necessary to protect human health and the environment;
- 3. Implement alternate measures for control of the sources of contamination, or for removal or decontamination of equipment, units, devices, or structures that are technically practicable and consistent with the overall objective of the remedy; and
- 4. Notify the IDNR within 14 days that a report justifying the alternate measures prior to implementation has been placed in the operating record.

Remediation will be considered complete when:

- 1. The owner or operator complies with the groundwater protection standards at all points within the plume of contamination that lie beyond the groundwater monitoring well system.
- 2. Compliance with the groundwater protection standards has been achieved by demonstrating that concentrations of Appendix II constituents have not exceeded the groundwater protection standard(s) for a period of 3 consecutive years. The IDNR may specify an alternative length of time during which the owner or operator must demonstrate that concentrations of Appendix II constituents have not exceeded the groundwater protection standard(s).
- 3. All actions required by the IDNR to complete the remedy have been satisfied.

Upon completion of the remedy, the owner or operator must notify the department within 14 days that a certification has been placed in the operating record verifying that the remedy has been completed. The certification must be signed by the owner or operator and by a qualified groundwater scientist and approved by the IDNR.

When, upon completion of the certification, the owner or operator determines that the corrective action remedy has been completed in accordance with IDNR regulations, the owner or operator will be released from the requirements for financial assurance for corrective action.

10 Annual Water Quality Reports

An annual report will be submitted to the IDNR in accordance with IDNR regulations in 567-113.10(10) detailing the water quality monitoring sampling locations and results, assessments, selection of remedies, implementation of corrective action, and the results of corrective action remedies to address SSLs, if any, during the previous year. The report will be due on March 31 of the current year for the previous monitoring year (i.e., March 31, 2025 for AWQR 2024).

The report will include:

- A site map delineating all monitoring points where water quality samples were taken.
- Map of contamination plumes, if any.
- A narrative explaining and interpreting all the data collected during the previous year.

Biennially, the report will also include monitoring well maintenance and performance evaluation information as detailed in Section 11.0. This was initiated with the 2013 AWQR. That information will include:

- Biennial evaluation of high and low water levels and their relationship to the well screen interval of individual wells.
- Biennial evaluation of Landfill operations and water level conditions to evaluate if there have been any changes to the hydrologic setting and resultant groundwater flow paths.
- Measurement of well depths to ensure that wells are physically intact and not filling with sediment.
- Biennial examination of well recharge rates and chemistry to determine if well deterioration is occurring.

11 Monitoring Well Maintenance and Performance Evaluation Plan

The monitoring well maintenance and evaluation plan for the Winneshiek County Landfill is designed to comply with IAC 567-113.10(455B)(2)f to ensure that all monitoring wells remain reliable. The plan includes:

- Physical examination of the well, protective casing, and upper well seal for well integrity
- Biennial evaluation of high and low water levels and their relationship to the well screen interval of individual wells.
- Biennial evaluation of landfill operations and water level conditions to evaluate if there have been any changes to the hydrologic setting and resultant groundwater flow paths.
- Measurement of well depths to ensure that wells are physically intact and not filling with sediment.
- Biennial examination of well recharge rates and chemistry to determine if well deterioration is occurring.

Monitoring well maintenance and evaluation has been reported in each year's Annual Water Quality Report since 2013.

11.1 Monitoring Well Maintenance and Performance Evaluation Reporting **Schedule**

Annual reports will be made of:

- The physical integrity of the wells.
- For wells without a dedicated pump, the evaluation of well depth and any indications that wells are filling with sediment will be completed annually. For wells with a dedicated pump, the evaluation will be made every 5 years.

Biennial reports will be made of:

- High and low water levels and their relationship as either above or within the well screen interval.
- Landfill operation activities that may have changed the hydrologic setting and the resultant groundwater flow paths.
- Evaluation of well recharge rates and chemistry to determine if well deterioration is occurring.

11.2 Physical Well Examination

The physical integrity of the wells will be evaluated semi-annually when water-level measurements are taken, and the wells are sampled. Notes will be made regarding:

- Protective casing status as locked or unlocked.
- Well cap condition.
- Presence of any obstructions in the well casing.
- Well casing integrity: presence or absence of cracks.
- Upper well seal integrity: intact or cracked.
- Any other items of note regarding well integrity.

Results of the physical well evaluation will be reported annually in the annual water quality report. If the integrity of any of the wells is in question, appropriate maintenance and rehabilitation procedures will be taken.

11.3 Evaluation of High and Low Water Levels and Their Relationship to the Well Screen Interval

The evaluation of water levels and their relationship to the well screen interval will be done to comply with IAC 567-113.10(455B)(2)f(1). Water levels will be measured to the nearest 1/100 of a foot as part of the semi-annual groundwater-level measurement program. The water-level depth will then be compared to the well screen depth to evaluate if well screens are placed appropriately in relationship to groundwater levels. Water table wells should have water levels within the well screen interval unless the water table is so shallow that it occurs in the upper seal interval. Deep wells should have water levels above the well screen interval. Results of the evaluation will be reported biennially in the AWQR for that year.

11.4 Evaluation of Landfill Operations on Groundwater Levels

To comply with IAC 567-113.10(455B)(2)f(2), landfill operations will be reviewed to determine if there have been any changes that would affect groundwater depth and flow. Such operational changes might include landfill excavation activities, installation of underdrain systems, etc. Results of the evaluation will be reported biennially in the AWQR for that year.

11.5 Evaluation of Well Depth Changes

Evaluation of the bottom of the well depth changes will be done to comply with IAC 567-113.10(455B)(2)f(3). In wells that do not have dedicated sampling pumps, the depth to the bottom of the well will be measured monthly to the nearest 1/100 foot using a weighted tape. For wells with dedicated sampling pumps, measurement of the depth to the bottom of the well will be made every 5 years.

For each well, comparison of the depth to the bottom of the well will be made over time to determine if sediment is building up in the well. If significant sediment build-up occurs, appropriate rehabilitation techniques, such as redevelopment by surging and bailing or other methodology, will be completed. If the monitoring well cannot be redeveloped or rehabilitated, the well will be abandoned and replaced. The results of the evaluation of well depth changes will be reported biennially in the AWQR for that year.

11.6 Evaluation of Well Recharge Rates and Groundwater Chemistry

To comply with IAC 567-113.10(455B)(2)f(4), well recharge rates and groundwater chemistry will be evaluated to determine if well deterioration is occurring.

Well recharge rates will be determined biennially during one of the groundwater sampling events. To estimate the recharge (recovery) rate, at least one, and preferably several, time, and depth to water (D.T.W.) measurements will be recorded after removal of the final purge volume to gauge recovery relative to the static water level. Recording the time, depth to water, and the volume removed during the purge and sampling cycle will provide for an estimate of well recharge (recovery) rates. If time permits, measurements will be taken until the well is fully recovered and the time that the well returned to static water level will be recorded. As an example:

- Depth to water (D.T.W.) is measured prior to purging/sampling (i.e., static water level) and is at 10 feet below ground surface.
- \bullet At the end of the purge/sample cycle, the measured D.T.W. $= 12$ feet below ground surface.
- \bullet 30 minutes later, D.T.W. = 10.5 feet below ground surface.
- Therefore, the well recovered 1.5 feet in 30 minutes.
- Considering that the gallons of water per foot in a 2-inch diameter well is 0.16 gal/ft results in an estimated recharge rate of 0.008 gpm ((1.5 ft x 0.16 gal/ft) / 30 min).

Estimates calculated in this manner for each sampling event can be used to determine if well recovery rates are decreasing over time, indicating well deterioration may be occurring because of infilling of the well with sediment, biofouling of the filter pack, etc.

In addition, detection monitoring wells which are sampled semi-annually may be analyzed using the slug test method once per 5 years. A slug test is an aquifer field test performed by groundwater hydrogeologists to estimate the hydraulic properties of aquifers and aquitards. The slug test is a method in which the water level in a monitoring well is caused to change suddenly (rise or fall), and the subsequent water-level response (displacement or change from static) is measured through time in the monitoring well. Other terms sometimes used instead of slug test include bail-down test, slug-in test, and slug-out test.

The goal of a slug test is to estimate hydraulic properties of a monitoring well's formation and development of the well's filter pack. A change in the hydraulic properties of the monitoring well's formation, the filter pack or well screen may indicate that that well deterioration is occurring because of infilling of the well with sediment, biofouling of the filter pack, etc.

Groundwater chemistry changes over time will also be evaluated to determine if there are any physical or chemical indicators affecting groundwater quality. Physical indicators include encrustation, biofouling, corrosion, and change in turbidity. Chemical indicators include changes in redox potential (Eh), pH, conductivity, and dissolved gases (such as oxygen, carbon dioxide, nitrogen, hydrogen sulfide, and methane) that may affect groundwater chemistry.

FIGURES

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Deep Well Water Elevations

Hydrologic Monitoring System Plan Winneshiek County Solid Waste Agency Decorah, Iowa

Figure 29 Landfill Base Grades

Hydrologic Monitoring System Plan Winneshiek County Solid Waste Agency Decorah, Iowa

March 2024

EIGURE 30
2024 HMSP FIGURE 30 Theoretical Release Model Domain

WINNESHIEK COUNTY LANDFILL

SEPTEMBER 2024

TABLES

Winneshiek County Landfill Hydrologic Monitoring System Plan

Notes:

¹ Screened geologic unit symbols:

Qf = Quaternary soil fill placed during landfill construction

Ql = Quaternary loess

Qc = Quaternary, pre-Wisconsin Episode colluvium

Qt = Quaternary pre-Illinoian-age diamicton ("till")

fluv = Quaternary fluvioglacial material

Oeu = Ordovician Maquoketa Formation, Upper Elgin Member (interbedded limestone and shale)

Oel = Ordovician Maquoketa Formation, Lower Elgin Member (shale)

Og = Ordovician Galena Group Decorah Formation (limestone)

 $NA = not$ applicable

Locations Included in Semi-Annual Groundwater, Surface Water, and Leachate Head Elevation Measurements Winneshiek County Landfill

Page 1 of 4 March

Locations Included in Semi-Annual Groundwater, Surface Water, and Leachate Head Elevation Measurements Winneshiek County Landfill

Locations Included in Semi-Annual Groundwater, Surface Water, and Leachate Head Elevation Measurements Winneshiek County Landfill

Notes:

¹ Screened geologic unit symbols:

Qal = Quaternary alluvium

Qf = Quaternary soil fill placed during landfill construction

Locations Included in Semi-Annual Groundwater, Surface Water, and Leachate Head Elevation Measurements Winneshiek County Landfill

Ql = Quaternary loess

Qc = Quaternary, pre-Wisconsin Episode colluvium

Qt = Quaternary pre-Illinoian-age diamicton ("Till")

fluv = Quaternary fluvioglacial material

Oeu = Ordovician Maquoketa Formation, Upper Elgin Member (interbedded limestone and shale)

Oel = Ordovician Maquoketa Formation, Lower Elgin Member (shale)

Og = Ordovician Galena Group Decorah Formation (limestone)

NA = not applicable

Compound	MDL (μ g/L)	RL (µg/L)	GPS (μ g/L)
Dibromochloromethane	0.20	5	80
Dibromomethane	0.18	1	70
Dichloromethane	0.17	5	5
Ethylbenzene	0.21	1	700
lodomethane	0.80	10	NS
Styrene	0.10	1	100
Tetrachloroethylene	0.18	1	5
Toluene	0.15	1	1000
Trans-1,2-Dichloroethylene	0.21	1	100
Trans-1,3-Dichloropropene	0.22	5	NS
Trans-1,4-Dichloro-2-butene	0.13	10	1.8
Tribromomethane	0.14	5	80
Trichloroethylene	0.19	1	5
Trichlorofluoromethane	0.17	$\overline{4}$	2000
Trichloromethane	0.28	1	80
Vinyl Acetate	0.74	10	NS
Vinyl Chloride	0.10	1	$\overline{2}$
Xylenes	0.13	3	10000

(Continued) List of Appendix I Analytical Parameters and Detention Limits Table 7

Notes:

NS = No standard

MDL = Method Detection Limit

RL = Reporting Limit

GPS = Groundwater Protection Standard

Appendix A – Boring Logs and Iowa DNR Monitoring Well / Piezometer Construction Documentation Forms (Form 542-1277)

Appendix

Appendix A-1 – 1973 – Boring Logs and Monitoring Well Construction Documentation

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Appendix A-2 – 1984 – Boring Logs and Monitoring Well Construction Documentation

Appendix A-3 – 1989 – Boring Logs and Monitoring Well Construction Documentation

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MONITORING WELL / PIEZOMETER CONSTRUCTION

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JAMES M. MONTGCMERY CONSULTING ENGINEERS, INC.

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Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street,
Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-

Revised 9/05

Form # 542-1277

ELEVATIONS: ± 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM
GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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MONITORING WELL / PIEZOMETER CONSTRUCTION

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> **URAN** - 4

MONITORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Disposal Site Name Winneshiek County Landfill

Permit No. #96-SDP-1-74P

Well or Piezometer No. $M/d - 2$

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.

Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

Form # 542-1277

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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MONITORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Permit $\frac{3}{7}$ 96 -SDP-1 - 74 P Disposal site name Winneshiek County Well or Pieromerer FMW-3 Date started 7-21-89 Date completed 7-21-89 A. Surveyed Locations and Elevations Locations $(\frac{1}{2} 0.5 \text{ ft.})$: Well Installation, continued: NW Specify corner of site Distance and direction Filter pack: along boundary Material Muscatine Sand Grain size \sharp 1 Distance and direction Volume from boundary to well 8' Fast and 330' south of NW Corner Seal (minimum 3 ft. length above $filter pack$: Elevations ($\stackrel{+}{\sim}$ 0.01 ft. MSL): Material Bentonite Pellets Ground surface 1160.84 Placement method Tremie Tube Top of protective casing 1163.34 Volume Top of well casing 1162.84 Benchmark elevation 1158.80 Backfill (if different from seal): Benchmark description Second snike Material Neat Cement Placement Method Tremie Tube in power pole at NW Corner of Site Volume **B. Soil Boring Information** Surface seal design: Name and address of construction Material of protective casing:
Steel company J&R Drilling Material of grout between protect-7922 N.W. 114th Grimes. IA 50011 ive casing and well casing: Mame of driller R Coons Kwikcrete Drilling method Continuous Flight Auger Protective cap: Drilling fluid Material Steel Bore hole diameter 6.00" Vented? Y/N Y Locking? Y/N Y Scil sampling method Split Spoon Well cap: **PVC** Material Depth of boring <u>251</u> Vented? Y/N Y C. Monitoring Well Installation D. Groundwater Measurement Casing material PVC. Water level $(1, 0.01$ ft. below top 36.91 Langth of casing Outside casing diameter of inner well casing) 1137.49 $21^{\frac{11}{2}}$ Stabilization time Inside casing diameter Well development methodAir Jetting Threaded Casing joint type Casing/screen joint type Threaded **PVC** Screen material Screen opening size $.010$ Upgradient or downgradient well? (see piezometric map from Hydro-Screen length 10^{1} Depth of well 36.9 geologic study) Average depth of frostline 3.0'

JAMES M. MONTGOMERY CONSULTING ENGINEERS, INC.

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: Iowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.

Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

Form #542-1277

ELEVATIONS: 1 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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MONETORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Permit $\frac{1}{2}$ 96-SDP-1-74P Disposal site name Winneshiek County Date started 7-20-89 Date completed 7-21-89 Well or Piezometer # MW-4

A. Surveyed Locations and Elevations

Locations $(\frac{+}{-} 0.5 \text{ ft.})$: Specify corner of site **NW** Distance and direction along boundary

Distance and direction from boundary to well 7' south
and 563' east of NW corner

Elevacions $(\pm 0.01$ ft. MSL): Ground surface £178.52 Top of protective casing 1181 02 Top of well casing <u> 1180.52</u> Benchmark elevation 1158.80 Benchmark description Second snike in power pole at Nw corner of site

3. Soil Boring Information

C. Monitoring Well Installation

Well Installation, continued:

- Seal (minimum 3 ft. length above filter pack): Material Bentonite Pellets Placement method Tremie Tube Volume
- Backfill (if different from seal): Material Neat Cement Placement Method Tremie Tube Volume
- Surface seal design: Material of protective casing:
Steel
	- Material of grout between protect. ive casing and well casing:
	- Kwikcrete Protective cap: **Steel** Material Locking? Y/N Y Vented? $\overline{Y/N}$ γ Well cap: **PVC** Material Vented? $\overline{Y/N}$ Y

D. Groundwater Measurement

Water level $(1, 0.01)$ ft. below top of inner well casing) 1141.13' Stabilization time Well development method Air Jetting

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) Average depth of frostline $3.0'$

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina koger@dnr.state.ia.us

Revised 9/05

Form # 542-1277

ELEVATIONS: 1 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM
GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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TOP OF THREADED PVC CAP $MW-4$ 2 FOOT MINIMUM ABOVE GRADE TOP OF PROTECTIVE CASING-**ELEVATION 1181.02** 6 INCH PEA GRAVEL AND COARSE SAND TOP OF WELL CASING -
ELEVATION 1180.52 SLOPE GROUNT AWAY FROM CASING GROUND SURFACE + 2 TO PREVENT INFILTRAION 110 St **ELEVATION 1178.52** GROUT SEAL TO SURFACE PROTECTIVE CASING 4 FOOT MINIMUM DEPTH TOP OF BACKFILL -BASE OF CONCRETE PLUG AND BENTONITE GROUT **ELEVATION 1174.02 DEPTH** 4.5 BASE OF PROTECTIVE CASING -ELEVATION 1175.02 **DEPTH** 3.5 BASE OF BACKFILL-TOP OF SEAL BENTONITE SEAL ELEVATION 1148.32 VOLCAY BENTONITE PELLETS (ADD WATER IF ABOVE WATER TABLE) TOP OF FILTER PACK-BASE OF SEAL **ELEVATION 1145.32** DEPTH 33.2 MUSCATINE FILTER PACK \mathcal{L} ABOVE TOP OF SCREEN TOP OF SCREEN-۰ \mathbb{R}^2 : ELEVATION 1143.82 DEPTH 34.7 PVC #10 SLOT WELL SCREEN BOTTOM OF SCREEN-ELEVATION 1133.82 **DEPTH** 44.7 **BASE OF FILTER PACK -**ELEVATION 1132.82 DEPTH 45.7

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MONITORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Permit \neq 96 -SDP-1 - 74 P Disposal site name Winneshiek County Date Started 7-20-89 Date completed 7-20-89 Well or Plezomerer ; MW-5

A. Surveyed Locations and Elevations

Locations $(\frac{1}{2}, 0.5, \frac{1}{2})$: Specify corner of site NE Distance and direction along boundary

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Distance and direction from boundary to well 398 Feet West and 335 feet south of NE corner

 E lavations ($\stackrel{+}{\sim}$ 0.01 ft. MSL): Ground surface 1155.93 Top of protective casing 1158 43 Top of well casing 1157.93 1158.80 Benchmark elevation Benchmark description Second spike in power pole at NW corner of site.

B. Soil Boring Information

Name and address of construction company_ J&R Drilling 7922 N.W. 114th Grimes, TA 50011 Mame of driller R. Coons
Drilling method Continuous Flight Auger Drilling fluid Bore hole diameter 6,00" Scil sampling method. Split Spoon Depth of boring 251

C. Monitoring Well Installation

Well Installation, continued:

- Seal (minimum 3 ft. length above $filter$ $pack$): Material Bentonite Pellets Placement method Tremie Tube Volume
- Backfill (if different from seal): Material Neat Cement Placement Method Tremie Tube Volume
- Surface seal design: Material of protective casing:
Steel

Material of grout between protective casing and well casing: Kwikcrete

Protective cap: Material Steel Locking? Y/N Y Vented? Y/N Y_{\perp} Well cap: **PVC** Material Vented? Y/N Y

D. Groundwater Measurement

Water level $(1, 0.01$ ft. below top of inner well casing) Dry Stabilization time Well development method

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) 3.0^+ Average depth of frostline

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Permit # $96 - SDP - 1 - 74P$ Oisposal site name Winneshiek County Well or Piezomerer # MW-6 Date started 7-20-89 Date completed 7-20-89

A. Surveyed Locations and Elevations

Locations $(-0.5 f_L)$: Specify corner of site_NE Distance and direction along boundary

Distance and direction from boundary to well 400 feet and 335 feet south of NE corner ...

Elevations $(\pm 0.01$ ft. MSL): 1156.04 Ground surface Top of protective casing 1158.54 Too of well casing 1158 04 Benchmark elevation 1158.80 Benchmark description Second spike in power pole at NW corner of site

B. Soil Boring Information

Name and address of construction company J&R Drilling 7922 N.W. 114th Grimes. IA 50011 Mame of driller R. Coons
Drilling method Continuous Flight Auger Drilling Eluid \sim $-$ Bore hole diameter 6,00" Scil sampling method Split Spoon Depth of boring 251

C. Monitoring Well Installation

Well Installation, continued:

Filter pack: Material Muscatine Sand Grain size $#1$ Volume

- Seal (minimum 3 ft. length above filter pack): Material Bentonite Pellets Placement method Tremie Tube Volume
- Backfill (if different from seal): Material Neat Cement Placement Method Tremie Tube Volume
- Surface seal design: Material of protective casing:
Steel
	- Material of grout between protect ive casing and well casing: Kwikcrete Protective cap:
	- Material Steel Vented? Y/N Y Locking? Y/N Y Nell cap: **PVC** Material Vented? Y/N Y

D. Groundwater Measurement

Water level (¹0.01 ft. below top of inner well casing) 1129.16 Stabilization time Well development method Air Jetting

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) Average depth of frostline 3.0°

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Permit $\frac{1}{2}$ 96-SDP-1- 74 P Disposal site name Winneshiek County Well or Piezomerer # MW-7 Date started 7-19-89 Date completed 7-20-89

A. Surveyed Locations and Elevations

Locations $(\pm 0.5$ ft.): Specify corner of site NE Distance and direction along boundary

Distance and direction from boundary to well 398 feet west and 880 feet south of NE corner

 \mathbb{E} levations (\mathbb{L} 0.01 ft. MSL): Ground surface 1177.18 Top of protective casing 1179.68 Top of well casing 1179 18 Benchmark elevation 1158.80 Benchmark description Second spike in power pole at NW corner of site

3. Soil Boring Information

20th

Name and address of construction company J&R Drilling 7922 N.W. 114th Grimes. IA 50011 Mame of driller R Coons Drilling method Continuous Flight Auger Drilling fluid --Bore hole diameter 6.00" Scil sampling method Split Spoon Depth of boring 251

C. Monitoring Well Installation

Well Installation, continued:

- Seal (minimum 3 ft. length above filter pack): Material Bentonite Pellets Placement method Tremie Tube Volume
- Backfill (if different from seal): Material Neat Cement Placement Method Tremie Tube Volume
- Surface seal design: Material of protective casing:
Steel
	- Material of grout between protective casing and well casing: Kwikcrete Protective cap:
	- Material Steel Vented? Y/N Y Locking? Y/N y Well cap: **PVC** Material Vented? $\overline{Y/N}$ \overline{Y}

D. Groundwater Measurement

Water level (¹0.01 ft. below top of inner well casing) Ury Stabilization time Well development method

Ucgradient or downgradient well? (see piezometric map from Hydrogeologic study) $3.0'$ Average depth of frostline

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Disposal site name Winneshiek County Permit $\frac{3}{2}$ 96 -SDP-1 - 74 P Well or Piezometer # MW-8 Date started 7-20-89 Date completed 7-20-89

A. Surveyed Locations and Elevations

Locations $(\frac{1}{2} 0.5 f_{2})$: **NE** Soecify corner of site Distance and direction along boundary

Discance and direction Erom boundary to well_398 feet west and 1115 feet south of NE corner

Elevations $(\pm 0.01$ ft. MSL): Ground surface 1162.52 Top of protective casing 1165.02 Too of well casing 1164.52 Benchmark elevation 1158.80 Benchmark description Second snike in power pole at NW corner of site

8. Soil Boring Information

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Name and address of construction company J&R Drilling 7922 N.W. 114th Grimes. IA 50011 Mame of driller R. Coons Drilling method Continuous Flight Auger Drilling fluid -Bore hole diameter 6,00" Scil sampling method Split Spoon Deoth of boring 251

C. Monitoring Well Installation

Well Installation, continued:

- Filter pack: Material Muscatine Sand Grain size $#1$ Voluma
- Seal (minimum 3 ft. length above filter pack): Material Bentonite Pellets Placement method Tremie Tube Volume
- Backfill (if different from seal): Material Neat Cement Placement Method Tremie Tube Volume
- Surface seal design: Material of protective casing:
Steel
	- Material of grout between protect. ive casing and well casing: Kwikcrete
	- Protective cap: Material Steel

Vented? Y/N Y Locking? Y/N Y Well cap: **PVC** Material Vented? Y/N Y

D. Groundwater Measurement

Water level $(\pm 0.01$ ft. below top of inner well casing) 1151.18 Stabilization time Well development method Air Jetting

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) Average depth of frostline 3.0'

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Permit $\frac{1}{2}$ 96-SDP-1-74 P Disposal site name Winneshiek County Well or Piezometer ; MW-9 Date started 7/17/89 Date completed 7/17/89 A. Surveyed Locations and Elevations Locations $(\pm 0.5 f t.)$: Well Installation, continued: Specify corner of site SE. Distance and direction Filter pack: along boundary Material Muscatine Sand Grain size #1 Distance and direction Volume from boundary to well 85' East
and 9' north of SE corner Seal (minimum 3 ft. length above Eilter pack): Elevacions $(\pm 0.01 \text{ ft.} \text{MSL})$: Material Bentonite Pellets Ground surface 1186,19 Placement method Tremie Tube Top of protective casing 1188 69 Volume

- Backfill (if different from seal): Material Neat Cement Placement Method Tremie Tube Volume
- Surface seal design: Material of protective casing:
Steel Material of grout between protect
	- ive casing and well casing: Kwikcrete Protective cap:
- Material Steel Vented? Y/N Y Locking? Y/N Y Well cap: **PVC** Material
	- Vented? Y/N Y
- D. Groundwater Measurement

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) ᠊ᢋᠽᡎ᠆ Average depth of frostline

JAMES M. MONTGOMERY

CONSULTING ENGINEERS, INC.

7922 N.W. 114th Grimes. IA 50011 Drilling method Continuous Flight Auger **PVC** Casing material 26.6 Length of casing Outside casing diameter $24"$ 2^{N} Inside casing diameter Casing joint type **Threaded** Casing/screen joint type_ Threaded **PVC** Screen material Screen opening size $.010$ Screen langth 10'

 $26.6'$

- Too of well casing 1188.19 Benchmark elevation 1158.80 Benchmark description Second spike in power pole at NW corner of site.
- 3. Soil Boring Information
- Name and address of construction company <u>J&R Drilling</u> Mame of driller R. Coons Drilling fluid Bore hole diameter 6,00" Soil sampling method Split Spoon Depth of boring 25!

C. Monitoring Well Installation

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Disposal site name Winneshiek County S.L.F. Permit # 96 -SDP- 1 - 74 P
Well or Piezomerer # MW-18 Date started 12-29-89 Date completed 12-29-89 Permit $# 96$ -SDP- $1 - 74$ P

A. Surveyed Locations and Elevations.

Locations $(\pm 0.5 \text{ ft.})$: **NW** Specify corner of site Distance and direction

Distance and direction from boundary to well 7.5' East

Elevations $(\pm 0.01$ ft. MSL): Ground surface 1161.32 Top of protective casing 1163.83 Top of well casing 1163.65 Benchmark elevation 1158.80 Benchmark description Second spike in power pole at NW corner of site.

B. Soil Boring Information

Name and address of construction J&R Drilling Services company 7922 NW 114th Grimes, IA 50011 Name of driller J. Stoy Drilling method Air Rotary Drilling fluid Air $\overline{A^{\mathrm{II}}}$ Bore hole diameter Soil sampling method Cuttings Depth of boring 45"

C. Monitoring Well Installation

Well Installation, continued:

Filter pack: Muscatine Sand Material Grain size #I 100 Lbs. Volume

- Seal (minimum 3 ft. length above filter pack): Material Bentonite Pellets Placement method Hand Poured Volume 50 Lbs.
- Backfill (if different from seal): Material Neat Cement Placement Method Tremie Tube Volume
- Surface seal design: Material of protective casing: Steel Material of grout between protective casing and well casing: Kwikcrete Protective cap: Steel Material Vented? Y/N Y Locking? Y/N Well cap: Material **PVC** Vented? $\overline{Y/N}$ γ

D. Groundwater Measurement

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) Downqradient Average depth of frostline

DES MOINES, IOWA

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: Iowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.

Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

Form # 542-1277

ELEVATIONS: 1 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

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SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

 $MW-18$ TOP OF PROTECTIVE CASING-**ELEVATION 1163.83** TOP OF WELL CASING -
ELEVATION 1163.65 GROUND SURFACE - A **ELEVATION 1161.32** 4 foot MINIMUM DEPTH TOP OF BACKFILL -BASE OF CONCRETE PLUG AND BENTONITE GROUT ELEVATION 1157.57 BASE OF PROTECTIVE CASING
ELEVATION 1158.83 V. BASE OF BACKFILL -TOP OF SEAL ELEVATION 1129.07 TOP OF FILTER PACK-**BASE OF SEAL** ELEVATION 1125.90 DEPTH 35.42 $15.$ TOP OF SCREEN-**ELEVATION 1123.65 DEPTH** 37.67 BOTTOM OF SCREEN-**ELEVATION 1118.65 DEPTH** 42.67 **BASE OF FILTER PACK -**
ELEVATION 1117.90
DEPTH 43.42

PROTECTIVE CASING

BENTONITE SEAL

MUSCATINE FILTER PACK ABOVE TOP OF SCREEN

Appendix A-4 – 1990 – Boring Logs and Monitoring Well Construction Documentation

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MONITORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Disposal Site Name Winneshiek County Landfill

Permit No. #96-SDP-1-74P

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.

Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

Form # 542-1277

ELEVATIONS: 1 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

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SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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 $MW-19$ TOP OF PROTECTIVE CASING-ELEVATION 1205.90 TOP OF WELL CASING -
ELEVATION 1205.65 GROUND SURFACE - 2 96.320 **ELEVATION 1203.61** l s TOP OF BACKFILL -BASE OF CONCRETE PLUG AND BENTONITE GROUT **ELEVATION 1199.11 DEPTH** 4.5 **BASE OF PROTECTIVE** CASING -ELEVATION 1199.90 **DEPTH** 3.71 BASE OF BACKFILL-TOP OF SEAL ELEVATION ... DEPTH_ × TOP OF FILTER PACK-**BASE OF SEAL** ELEVATION 1154.83 DEPTH 48.78 TOP OF SCREEN-ELEVATION 1151.83 **DEPTH** 51.78 **BOTTOM OF SCREEN-
ELEVATION** 1131.83
DEPTH 71.78 **BASE-OF FILTER PACK -**
ELEVATION 1130.83
DEPTH 72.78 DEPTH

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PROTECTIVE CASING

BENTONITE SEAL

MUSCATINE FILTER PACK ABOVE TOP OF SCREEN

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MONITORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Disposal site name Winneshiek County
Well or Piezometer # MW-19 Date started 8-16-90 Date completed 8-22-90

A. Surveyed Locations and Elevations

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Locations $(\frac{r}{r}, 0.5 f_{r})$: **SW** Specify corner of site Distance and direction along boundary 259.4' East

Distance and direction from boundary to well 11.6 Feet North

Elevations $(\pm 0.01$ ft. MSL): Ground surface 1203.61 Top of protective casing 1205.90 Top of well casing 1205.65 Benchmark elevation 1158.80 Benchmark description Second spike in power pole at NW corner of site

B. Soil Boring Information

Name and address of construction company Shawver Well Co. Box 266 Fredericksburg. IA 50630 Name of driller Jim Bunting Drilling method Air Rotary Drilling fluid Air $77/8"$ Bore hole diameter Soil sampling method Recirculated Cuttings -173 Depth of boring

C. Monitoring Well Installation

Well Installation, continued:

Filter pack: Material Muscatine Sand Grain size #1 5.98 cubic feet Volume

- Seal (minimum 3 ft. length above filter pack): Material Bentonite Grout Placement method Tremie Tube
Volume 11.44 cubic feet
- Backfill (if different from seal): Material As Above Placement Method Volume

D. Groundwater Measurement

Water level $(1, 0.01)$ ft. below top of inner well casing) 49.63 Stabilization time 24 Hours Well development method Bailing

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) Upgradient Average depth of frostline 3.0'

> JAMES M. MONTGOMERY CONSULTING ENGINEERS, INC.

> > APPENDIX E

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APPENDIX I

MONITORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Permit # $96 - SDP - 1 - 74$ Disposal site name Winneshiek County Well or Piezometer # MW-20 Date started 8-28-90 Date completed 8-28-90

A. Surveyed Locations and Elevations

Locations $(\frac{r}{r} \ 0.5 \ f t.)$: Specify corner of site NW Distance and direction along boundary 294.5 Feet South

Distance and direction from boundary to well 7.5 Feet East

Elevations $(\frac{+}{-} 0.01$ ft. MSL): Ground surface 1160.16 Top of protective casing 1162.24 Top of well casing 1161.99 Benchmark elevation 1158.80 Benchmark description Second spike in power pole at NW corner of site

B. Soil Boring Information

Name and address of construction company Shawver Well Co. Box 266 50630 <u>Fredericksburg, IA</u> Name of driller Jim Bunting Drilling method Air Rotary Drilling fluid Air $77/8"$ Bore hole diameter Soil sampling method Recirculated Cuttings Depth of boring 111 Feet

C. Monitoring Well Installation

Well Installation, continued:

- Filter pack: Muscatine Sand Material H Grain size Volume 3.25 Cubic Feet
- Seal (minimum 3 ft. length above $filter$ $pack$): Material Bentonite Grout Placement method Tremie Tube Volume 14.30 Cubic Feet

D. Groundwater Measurement

Water level $(1, 0.01)$ ft. below top of inner well casing) 70.39 Stabilization time 24 Hours Well development method Bailing

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) Downgradient Average depth of frostline 3.0'

> JAMES M. MONTGOMERY CONSULTING ENGINEERS, INC.

APPENDIX E

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APPENDIX

MONITORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Permit $\frac{1}{2}$ 96 - SDP-1 - 74 Disposal site name Winneshiek County Well or Piezometer # MW-21 Date started 8-23-90 Date completed 8-27-90

A. Surveyed Locations and Elevations

Locations ($-$ 0.5 ft.): Specify corner of site_NE Distance and direction along boundary 381.4 Feet West

Distance and direction from boundary to well 335 Feet South

Elevations $(\pm 0.01 \text{ ft.} \text{MSL})$:
Ground surface 1156.56 Top of protective casing 1158.48
Top of well casing 1158.23 Benchmark elevation 1158.80 Benchmark description Second spike in power pole at NW corner of site

B. Soil Boring Information

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Name and address of construction company Shawver Well Co. Box 266 Fredericksburg. IA 50630 Name of driller Jim Bunting
Drilling method Air Rotary
Drilling fluid Air $7\frac{7}{8}$ Bore hole diameter Soil sampling method Recirculated Cuttings
Depth of boring 109' (2001)

C. Monitoring Well Installation

Well Installation, continued:

Seal (minimum 3 ft. length above filter pack): Bentonite Grout Material Placement method Tremie Tube
Volume 12.48 Cubic Feet

D. Groundwater Measurement

Water level $(1, 0.01)$ ft. below top of inner well casing) 56.42 Stabilization time 24 Hours Well development method Bailing

Upgradient or downgradient well? (see piezometric map from Hydrogeologic study) Downgradient Average depth of frostline 3.0'

> **JAMES M. MONTGOMERY** CONSULTING ENGINEERS, INC.

> > **APPENDIX E**

TABLE 3-3

LEACHATE HEAD ELEVATIONS

Appendix A-5 – 1993 – Boring Logs and Monitoring Well Construction Documentation

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Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: Iowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, <u>nina koger@dnr.state.ia.us</u>

Revised 9/05

Form # 542-1277

ELEVATIONS: 1 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM
GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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450 FROM MWH field boring log oject Winneshick County Landfiel (Rust E } Boring No. MWII-I Date Started 5/12/93 Date Complete 5/12/93 Drilled by Veff Logged by Scott Rig ORV water levels subsurface stratigraphy $[O^{'}$ While Drilling Da" Flight Augers △42" ID H.S. □62" ID H.S. From To Description. 0 Hours A.B. \mathcal{Z} Dk. brown salty clay +/ organics \circ 6.5 Hr. A.B. \mathcal{Z} Yell brn gray mott sid an set \sim_{c} l Gray rust brown mottled clancy silt 6.5 8 well details (t. gray brown mottled clayer silt \mathcal{E} $12,5$ **X**Stick-up Cover O Flush Cover $12,5$ 16 Rust brown claner silt 16 17 \mathcal{Z}' -47.3 Gray clanger silt is him sand 20.5 17 $\frac{\phi}{\phi}$ Grade $\frac{\phi}{\phi}$ ray far cla 2o.S BOB \mathfrak{S}' 25 Bottom of Boring sample data Depth Number/Type Depth Number/Type $7'$ $1 - C5$ $0 - 5$ $5 - 10$ $2-c5$ $10 - 15$ $3-c5$ $9¹$ $15 - 20$ $4-c5$ $20 - 25$ $5 - C5$ $\tilde{\mathcal{B}}$ $24'$ \overline{C} - Continuous Sampler $\overline{\Lambda}$ $\overline{\Lambda}$ $\overline{\Lambda}$ $\overline{\Lambda}$ $\overline{\Lambda}$ Auger Sample aquadr

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street,
Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-

Revised 9/05

Form #542-1277

ELEVATIONS: ± 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

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Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-

Revised 9/05

Form # 542-1277

ELEVATIONS: 1 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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1030 FRON MW4 INCINE W/MNIT-2 field boring log OJECT WINDESHIELL CO SLF Boring No. MW II 3 Date Started 5 13 93 Date Complete 5 13 93 $Drilled by $\frac{pFf}{pF}$ Logyed by $\frac{fQf}{f}$ Rig $GRV$$ water levels subsurface stratigraphy 3.0 While Drilling E4%" ID H.S. 06%" ID H.S. Da" Flight Augers $From$ Description T_{Ω} 0 Hours A.B. YELLOW BROWN CLAYEY SILTIS/ORGANICS $O \cdot O$ OS $Hr. A.B.$ DARK BROWN SILTY CLAY W /ORGANICE- $0.5 - 3.5$ Lt Corry Brown MOTHED CIRYEY SILT $35 - 70$ well details yerrow Brownlared montan Sity City w/statistick tip cover $10 - 12.5$ O Flush Cover youar Braez Sict Stant 10.5 11.5 130 GREENISH GREY SHALE $3.01129.6$ 11.5 YELLOW BROWN WEATHERED LIMESTUNE 13.0 $\frac{\phi}{\phi}$ Grade $\frac{\pi}{\sqrt{2}}$ 5.0 * SAMPLER REFUSAL @ 12.5 * AUGGER REFUSAL O 14.5 HOLE Bottom of Boring COOT 145 RLIG sample data Depth Number/Type Depth Number/Type 7.O $1CS$ 0050 $50,002$ 208 1000 ABODe QZQS. 9.O 10.0 12.5 30.8 \widetilde{S}

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 $\overline{\text{AS}}$ Augen Sumple I Continuous Sampler CS.

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina koger@dnr.state.ia.us

Revised 9/05

ELEVATIONS: 1 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM
GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: Iowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Envir

Revised 9/05

Form # 542-1277

ELEVATIONS: ± 0.01 FT. MSL

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

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Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Envir

Revised 9/05

Form # 542-1277
ELEVATIONS: ± 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM
GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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 $780'$ FROM NW 4 field boring log Project WINNESHIER CO SUF Boring No. Mulli'Q Date Started 5 11 93 Date Complete 5 11 93 Drillad by SEFF Logged by Scott ... Rig ORV subsurface stratigraphy water levels \Box 4" Flight Augers \Box 4" ID.H.S. 9.0 While Drilling \Box 6%" IO H.S. . From T_{α} . Description 40^t Hours A.B. MOD BROWN BILTY CLAY W JORGANICS $\overline{O.O}$ \overline{O} \overline 1.0 4.5" BROOD SILTY CLAY = ORTANCS Hr. A.B. 45: 145 L'EREX BROWN MOTHED CLAVE Y SILT/FINES **ANDI** details 145 100 yerror Brown SIGY SANZ FINE-MED erstick-up Cover 160 175 GREY OGREEN DOORED CLAVEY SILTY SAND Oflush Cover 175 180 GIEY EINE SICTY SAND 3.0 //52.9= $\overline{\mathscr{Q}}$ Grade $\overline{\mathscr{W}}$ DARX GREY SHALE 50 HOLE Bottom of Boring ZS.O PLUG sample data Depth Number/Type Depth Number/Type iCS $0.0 - 5.0$ 7.0 $cc\delta$ 50.42 තිරන $10.0.150$ $4/8$ 150.200 9.0 200-250 ්රයි HOCC ? CS - Continuous Sampler \overline{X} \overline{S} $\overline{$ August Sumple aquadr

Appendix A-6 – 1995 – Boring Logs and Monitoring Well Construction Documentation

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

Form #542-1277

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.

Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

Form #542-1277

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

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Appendix A-7 – 1997 – Boring Logs and Monitoring Well Construction Documentation

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.

Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

Form # 542-1277

ELEVATIONS: 1 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

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SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Please mail completed form to: Iowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina koger@dnr.state.ia.us

Revised 9/05

Form # 542-1277

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Please mail completed form to: Iowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.

Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

Form # 542-1277

ELEVATIONS: 1 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Envir

Revised 9/05

Form # 542-1277

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM
GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

Appendix A-8 – 2001 – Boring Logs and Monitoring Well Construction Documentation

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Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street,
Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-

Revised 9/05

Form # 542-1277

ELEVATIONS: ± 0.01 FT. MSL

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DEPTHS: ± 0.1 FT. FROM
GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Tracking Codes: 37655WIN.GPJ, ETROCK, 6/14/02, 12:12

Tracking Codes: 37655WIN.GPJ, ETROCK, 6/14/02, 12:12

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Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/₂ inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-281-8986, nina.koger@dnr.state.ia.us

Revised 9/05

ELEVATIONS: 1 0.01 FT. MSL

DEPTHS: ± 0.1 FT. FROM
GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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Sheet 5 of 7 EARTH T E C H **FIELD ROCK CORE LOG BORING NO.** A **tyco** INTERNATIONAL LTD. COMPANY PROJECT NO. 37655. 1050 SITE: Winnesheik County LandCII $MN-12$ **CORING DATA** % RECOVERY ROCK TYPE
CODE Depth GRAPHICS % ROD
FRAC./ FT **ROCK DESCRIPTION METHOD** RUN NO
LENGTH in **COMMENTS** Feet 66 NQ (3) 2.25 LS As Above From 65.25 FECT TO 66.0 FEET. GALENA · PLATTEVILLE (DOLOMITE) FORMATION. GAL WATER LOSS 67 Hard, White (7.5YR 8/1) DoLOMITIC 24 LIMESTONE, Moist to Dry Rock
Matrix; Fresh; Microcrystalline; 68 \mathbf{r} Interclastic; Medium to Thick Bedded; 69 $\overline{2}$ Horizontal *Undulatory* Beds; Trace
Horizontal Tosnts (Micro); Trace PER $70-$ Sciendary Pyrite Nidoles; Trace 100 **MINUTES** Vugs up to 1.5 cm d: amter filled 21 with calcite crystals jolnterclasts
Consist of Light Trace
Gray (7.58R 71.) Medium Grained APPROVIMATELY つと Ņ $\overline{5}$ Dolomitic Limestone 73 74 75 $rac{10}{120}$ R OD = $116" / 120"$ = $972'$ R_{un} 13 (97) loox' 76 &cont&

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VIL, F_ROCEP3, 3/8/94

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Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/₂ inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: Iowa Department of Natural Resources, Energy and Waste Management Bureau, 502 E. 9th Street, Des Moines, IA 50319-0034.
Des Moines, IA 50319-0034.
Questions? Call or Email: Nina Koger Envir

Revised 9/05

Form # 542-1277

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

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DTB 27' DTW 4' DTB-DTW= 23' 7 10ft below weterfalsh

PTB 21' DTW 8.5' D7B-DTW = 12ft 7 10ft below

Appendix A-9 – 2010 – Boring Logs and Monitoring Well Construction Documentation

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 $MW-12A$

WELL 13107005.GPJ TERRACON.GDT 5/26/10

 $MW-1$

LOG OF BORING NO. MW-1R Page 1 of 1 CLIENT
Winneshiek County Solid Waste Agency ENGINEER **AECOM SITE PROJECT Winneshiek County Sanitary Landfill Winneshlek County, Iowa SAMPLES TESTS** WELL **DETAIL DESCRIPTION** USCS SYMBOL RECOVERY, in. **GRAPHIC LOG** WATER
CONTENT, % DRY UNIT WT
pcf **BOREHOLE DIA.:** 7.25 In SPT-N"
BLOWS/ft DEPTH, fL **NUMBER WELL DIA.:** $2 \ln$ TOP OF PROTECTOR PIPE: $¹$ </sup> TYPE TOP OF CASING: $\ddot{}$ **GROUND SURFACE ELEV.:** \overline{c} $\overline{\text{ss}}$ ĩ $\overline{15}$ $\overline{4}$ **SANDY LEAN CLAY,** 3 **Dark Brown** $\overline{\text{CL}}$ $\overline{2}$ $\overline{\text{ss}}$ $\overline{20}$ $\overline{4}$ **LEAN CLAY, TRACE SAND,** $\boldsymbol{4}$ **Light Brown** $\overline{\text{CL}}$ $\overline{3}$ $\overline{\text{ss}}$ $\overline{12}$ $\overline{4}$ 5 $\overline{5}$ 5.5 $4UCH$ 4 $\overline{\text{ss}}$ 19 $\overline{2}$ 3 **QUCH 5** $\overline{\text{ss}}$ $\overline{\mathbf{3}}$ $\overline{16}$ 4 **LEAN TO FAT CLAY, TRACE SAND, Brown to Brown Gray** 10 $-GL/CH$ 6 $\overline{\text{ss}}$ $\overline{3}$ 18 3 $\overline{2}$ **QUCH7** $\overline{\text{ss}}$ 19 3 $\overline{\mathbf{3}}$ $-$ GL/CH $_8$ SS 21 15 6 $\overline{9}$ $\overline{9}$ $\overline{23}$ **CL** SS SANDY LEAN CLAY, TRACE GRAVEL, 11 **Brown** 24 18.5 $C-L - 10 - S$ -8 $\overline{\mathcal{A}}$ **BOTTOM OF BORING** The well consists of 2" diameter 0.010" PVC screen with a bottom point and solid PVC riser pipe to the surface. The annulus
was filled with sand to about 1' above the screen and the remainder with bentonite to the surface. The stratification lines represent the approximate boundary lines *Calibrated Hand Penetrometer "CME 140 lb. SPT automatic hammer between soll and rock types: In-situ, the transition may be graduat. **BORING STARTED** $4 - 22 - 10$ WATER LEVEL OBSERVATIONS, ft **Z NONE** WD $\sqrt{2}$ **WL BORING COMPLETED** 4-22-10 rraco ĮĪ $\overline{\mathbf{Y}}$ **WL RIG** 83E **FOREMAN** MW **WL DCC** 13107005 **APPROVED** JOB #

5/26/10 TERRACON, GDT $\overline{6}$ 13107005

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Sheet 2 of 2

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Appendix A-10 – 2015 – Boring Logs and Monitoring Well Construction Documentation

Tracking Codes: 60322851 GPJ, ETSOIL, 1023/19, 15:24

Tracking Codes: 60322851.GPJ, ETROCK, 10/23/19, 15:33

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Tracking Codes: 60322851.GPJ, ETROCK, 10/23/19, 15:33

Tracking Codes: 60322851.GPJ, ETSOIL, 10/23/19, 15:24

Tracking Codes: 60322851.GPJ, ETROCK, 10/23/19, 15:46

Continued Next Page

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Tracking Codes; 60322851.GPJ, ETSOIL, 10/23/19, 15:25

Tracking Codes: 60322851 GPJ, ETROCK, 10/23/19, 15:37

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Form 130

Appendix A-11 – 2016 – Boring Logs and Monitoring Well Construction Documentation

Tracking Codes: 60322851_43.GPJ, ETSOIL, 10/23/19, 16:18

Tracking Codes: 60322651_43.GPJ, ETSOIL, 10/23/19, 16:18

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/₂ inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Land Quality Bureau, 502 E. 9th St, Des Moines, IA 50319.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-725-8309, <u>nina koger@dn</u>

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM

GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

Please mail completed form to: lowa Department of Natural Resources, Land Quality Bureau, 502 E. 9th St, Des Moines, IA 50319. Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-725-8309, nina koger@dnr.iowa.gov

Tracking Codes: 60322851_43.GPJ, ETSOIL, 10/23/19, 16:18

Tracking Codes: 60322651_43 GPJ, ETSOIL, 10/23/19, 16:18

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 ½ inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Land Quality Bureau, 502 E. 9th St, Des Moines, IA 50319.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-725-8309, <u>nina koger@dn</u>

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM **GROUND SERFACE**

Please mail completed form to: Iowa Department of Natural Resources, Land Quality Bureau, 502 E. 9th St, Des Moines, 1A 50319. Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-725-8309, nina koger@dnr.iowa.gov

Tracking Codes: 60322851_43.GPJ, ETSOIL, 10/23/19, 16:28

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Tracking Codes: 60322851_43.GPJ, ETSOIL, 10/23/19, 16:28

Tracking Codes: 60322851_43.GPJ, ETROCK, 10/23/19, 16:17

Tracking Codes: 60322851_43.GPJ, ETROCK, 10/23/19, 16:17

Tracking Codes: 60322651 43.GPJ, ETROCK, 10/23/19, 16:17

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: lowa Department of Natural Resources, Land Quality Bureau, 502 E. 9th St, Des Moines, IA 50319.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-725-8309, <u>nina koger@dn</u>

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM GROUND SERFACE

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

Please mail completed form to: lowa Department of Natural Resources, Land Quality Bureau, 502 E. 9th St, Des Moines, IA 50319. Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-725-8309, nina koger@dnr.iowa.gov

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 1/2 inch x 11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed form to: Iowa Department of Natural Resources, Land Quality Bureau, 502 E. 9th St, Des Moines, IA 50319.
Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-725-8309, <u>nina koger@dn</u>

ELEVATIONS: ± 0.01 FT. MSL DEPTHS: ± 0.1 FT. FROM **GROUND SERFACE**

SPACE TO ATTACH ENTIRE SOIL BORING LOG (SHOW SCREENED INTERVAL AND FILTER PACK INTERVAL).

Please mail completed form to: Iowa Department of Natural Resources, Land Quality Bureau, 502 E. 9th St, Des Moines, IA 50319. Questions? Call or Email: Nina Koger Environmental Engineer Sr., 515-725-8309, nina koger@dnr.jowa.gov

Appendix A-12 – 2019 – Boring Logs and Monitoring Well Construction Documentation

FINAL ROCK CORE LOG

BORING NO.

S NO.
B-30 (Rock)
SHEET $\begin{array}{r} 2$ of 2 **B-30 (Rock)**

Tracking Codes: 60540571.GPJ, ETSOIL, 10/4/19, 10:20 Tracking Codes: 60540571.GPJ, ETSOIL, 10/4/19, 10:20

FINAL SOIL BORING LOG

BORING NO.

B-30

Tracking Codes: 60540571.GPJ, ETSOIL, 10/4/19, 15:38 Tracking Codes: 60540571.GPJ, ETSOIL, 10/4/19, 15:38

Tracking Codes: 60540571.GPJ, ETSOIL, 10/4/19, 15:39 Tracking Codes: 60540571.GPJ, ETSOIL, 10/4/19, 15:39

FINAL SOIL BORING LOG

BORING NO.

B-32

FINAL SOIL BORING LOG

BORING NO.

B-33

Tracking Codes: 60540571.GPJ, ETSOIL, 10/4/19, 15:40 Tracking Codes: 60540571.GPJ, ETSOIL, 10/4/19, 15:40

FINAL SOIL BORING LOG

BORING NO.

B-34

FINAL ROCK CORE LOG

BORING NO.

B-36 (Rock)

DL ORDOVICIAN GALENA GROUP DECORAH FM.; DOLOMITE (DL); light 3 -100% 83% <u>/ /</u> 100 3 - 100 \$ 73 % | / / 3 - 100% 65% 5.0 | 100 | 3 | 10 5.0 5.0 | 100 | 3 | 10 $11 \mid 5.0$ $12 \ 5.0$ $13 \ 5.0$ 65% slightly decomposed; slightly disintegrated; slightly to moderately fractured; conformable; with few pyrite infilled vugs as above from 70 - 75 ft bgs. brownish gray (5 YR 6/1) to bluish gray (5B 5/1); weak to moderate; microcrystalline; medium bedded; moderately to highly decomposed; moderately to intensely disintegrated; moderately to intensely fractured; conformable; numerous solution fractures; some infilled with clay as above from 80 - 85 ft bgs. EOB at 85 ft bgs.; air rotary overdill borehole and install monitoring well MW-36. Depth $\begin{bmatrix} \n\text{in} \\ \n\text{in} \\ \n\text{R} \\ \n\text{G} \n\end{bmatrix}$ and $\begin{bmatrix} \n\text{S} \\ \n\text{S} \\ \n\text{S} \\ \n\text{S} \n\end{bmatrix}$ Feet Tracking Codes: $\frac{1}{100}$, $\frac{1}{100}$ 70 75 80 85 B $\frac{1}{10}$ and $\frac{1}{10}$ b $\frac{1}{10}$ control c ROCK DESCRIPTION SITE: <u>Winneshiek County Landfill</u> PROJECT NO. 60540571 SHEET 3 OF 3 FRACTURES
ROCK
ROCK TYPE
ROCK TYPE CORING DATA $\frac{\mathsf{B-36 (Rock)}}{\mathsf{SHEET}}$ 3 of 3 67 % RQD
FRACTU
ROCK
TYPE GRAPHIC LOG

91

BORING NO.

B-36

Appendix A-13 – 2021 – Boring Logs and Monitoring Well Construction Documentation

BORING NO.

B-19R

BORING NO.

SHEET <u>2 OF 2</u> **B-19R (Rock)**

BORING NO.

B-37A

BORING NO.

B-38A

Tracking Codes: 60635552_MW.GPJ, ETSOIL, 10/20/21, 13:29 Tracking Codes: 60635552_MW.GPJ, ETSOIL, 10/20/21, 13:29

Tracking Codes: 60635552_MW.GPJ, ETSOIL, 10/20/21, 13:29 Tracking Codes: 60635552_MW.GPJ, ETSOIL, 10/20/21, 13:29

FINAL SOIL BORING LOG

BORING NO.

B-38B

BORING NO.

B-39A

Continued Next Page

Tracking Codes: 60635552_MW.GPJ, ETSOIL, 10/20/21, 13:29 Tracking Codes: 60635552_MW.GPJ, ETSOIL, 10/20/21, 13:29

FINAL SOIL BORING LOG

BORING NO.

B-39B

Appendix A-14 – 2022 – Boring Logs and Monitoring Well Construction Documentation

3105 Capital Way Cedar Falls, IA 50613 **P** (319) 277-4016 **F** (319) 277-4320 **Terracon.com**

Date: August 18, 2022

Winneshiek County Solid Waste Agency

201 West Main Street

Decorah, Iowa 52101

Attention: TJ Schissel

Re: Winneshiek County Landfill

Job No.: 13227047

Remarks:

If you have any questions regarding this information, please contact our office.

Yours truly,

By:

Terracon Consultants, Inc.

Dave Cleary

Dave Cleary

Copies to: Address (email) Russ Henning, AECOM, (email) Chris Oelkers, AECOM, (email)

IOWA DEPARTMENT OF NATURAL RESOURCES Abandoned Water Well Plugging Record

IOWA DEPARTMENT OF NATURAL RESOURCES Abandoned Water Well Plugging Record

IOWA DEPARTMENT OF NATURAL RESOURCES Abandoned Water Well Plugging Record

BORING NO.

B-41

FINAL SOIL BORING LOG

BORING NO.

B-42

Tracking Codes: 60635552_MW.GPJ, ETSOIL, 12/23/22, 10:19 Tracking Codes: 60635552_MW.GPJ, ETSOIL, 12/23/22, 10:19

IOWA DEPARTMENT OF NATURAL RESOURCES

MONITORING WELL/PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 ½x11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed for to: Iowa Department of Natural Resources, Land Quality Bureau, 502 E 9th St, Des Moines IA 50319-0034.

Questions? Call or Email: Nina Koger, Environmental Engineer Sr., 515-281-8986, Nina.Koger@dnr.iowa.gov

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ELEVATIONS: ± 0.01 ft MSL DEPTHS: ± 0.1 ft FROM GROUND SURFACE

IOWA DEPARTMENT OF NATURAL RESOURCES

MONITORING WELL/PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 ½x11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed for to: Iowa Department of Natural Resources, Land Quality Bureau, 502 E 9th St, Des Moines IA 50319-0034.

Questions? Call or Email: Nina Koger, Environmental Engineer Sr., 515-281-8986, Nina.Koger@dnr.iowa.gov

 $\overline{}$

ELEVATIONS: ± 0.01 ft MSL DEPTHS: ± 0.1 ft FROM GROUND SURFACE

IOWA DEPARTMENT OF NATURAL RESOURCES

MONITORING WELL/PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 ½x11 inch map showing locations of all monitoring wells and piezometers.

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 $\overline{}$

ELEVATIONS: ± 0.01 ft MSL DEPTHS: ± 0.1 ft FROM GROUND SURFACE

IOWA DEPARTMENT OF NATURAL RESOURCES

MONITORING WELL/PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

Attachments: Driller's log. Pipe schedules and grouting schedules. 8 ½x11 inch map showing locations of all monitoring wells and piezometers.

Please mail completed for to: Iowa Department of Natural Resources, Land Quality Bureau, 502 E 9th St, Des Moines IA 50319-0034.

Questions? Call or Email: Nina Koger, Environmental Engineer Sr., 515-281-8986, Nina.Koger@dnr.iowa.gov

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ELEVATIONS: ± 0.01 ft MSL DEPTHS: ± 0.1 ft FROM GROUND SURFACE

Appendix A-15 – 2023 – Boring Logs and Monitoring Well Construction Documentation

FINAL SOIL BORING LOG

BORING NO.

B-45

Tracking Codes: 60635552_MW.GPJ, ETSOIL, 03/26/24, 09:11 Tracking Codes: 60635552_MW.GPJ, ETSOIL, 03/26/24, 09:11

¹ Refer to the site's permit to determine applicable requirements. Note that some sites may only be regulated by their permit versus current landfill chapters. If the permit and rule are silent regarding applicable requirements, then 567 IAC Chapter 39 shall apply, which requires use of the Well Log (Well Record) Form, not this form. If the applicable requirements have been modified and approved by the DNR, then note under Other.

² The location does not need to be surveyed by a licensed surveyor. A handheld GPS reading accurate to +/- 30 feet is acceptable when an aerial photograph showing the location (pin) is included with this form. The site coordinates should be the same coordinate system currently used for survey control and mapping of the site.

DRILLER'S CERTIFICATION I certify under penalty of law I believe the information reported above is true, accurate, and complete.

Elevations: ±0.01 ft. MSL Depths: ±0.1 ft from Ground Surface

- Elevations for A, B, and C shall be surveyed.
- Depths for W, X, Y, and Z shall be field measured following completion of each item.
- Lengths of the Protective Casing (PC), Screen (S), and Well Point (WP) shall be field measured prior to installation of each item.
- The total Depth (TD) from the Top of Well Casing to the Bottom of Well Point shall be field measured following installation.

MONITORING WELL / PIEZOMETER CONSTRUCTION DOCUMENTATION FORM

 1 Refer to the site's permit to determine applicable requirements. Note that some sites may only be regulated by their permit versus current landfill chapters. If the permit and rule are silent regarding applicable requirements, then 567 IAC Chapter 39 shall apply, which requires use of the Well Log (Well Record) Form, not this form. If the applicable requirements have been modified and approved by the DNR, then note under Other.

² The location does not need to be surveyed by a licensed surveyor. A handheld GPS reading accurate to +/- 30 feet is acceptable when an aerial photograph showing the location (pin) is included with this form. The site coordinates should be the same coordinate system currently used for survey control and mapping of the site.

DRILLER'S CERTIFICATION I certify under penalty of law I believe the information reported above is true, accurate, and complete.

Elevations: ±0.01 ft. MSL Depths: ±0.1 ft from Ground Surface

- Elevations for A, B, and C shall be surveyed.
- Depths for W, X, Y, and Z shall be field measured following completion of each item.
- Lengths of the Protective Casing (PC), Screen (S), and Well Point (WP) shall be field measured prior to installation of each item.
- The total Depth (TD) from the Top of Well Casing to the Bottom of Well Point shall be field measured following installation.

3105 Capital Way Cedar Falls, IA 50613 **P** (319) 277-4016 **F** (319) 277-4320 **Terracon.com**

Date: November 13, 2023

Winneshiek County Solid Waste Agency

201 West Main Street

Decorah, Iowa 52101

Attention: TJ Schissel

Re: Winneshiek County Landfill

Project No. 13237075

Remarks:

If you have any questions regarding this information, please contact Dave Cleary at 319-277-4016. Thank you

Yours truly,

Terracon Consultants, Inc.

By:

Dave Cleary

Dave Cleary

Copies to: Address (email) Mr. Russ Henning, AECOM (email)

1. Owner:

1. Owner:

Appendix B – Example Field Water Elevation Form

Soil Conditions:

Water Elevation

Project No.: Site:

Appendix C – Example Well Purging Form

Field Meter Instrument Calibration Log

Project No.: Site: Date:

Name / Signature of personnel conducting calibration:

Comments:

Rev. 12/17/2019 F523/Earth.Sci

<u> 1980 - Johann Stoff, deutscher Stoffen und der Stoffen und der Stoffen und der Stoffen und der Stoffen und de</u>

Well Purging and Sampling Collection

Signature: Date:

Observe Conditions at the Well

Well ID: _____________________ Date: _________________

The following checklist should be used to assess well integrity. A checkmark for each item signifies completion; comments should be added if appropriate.

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Physical surroundings (e.g. high vegetation, standing water, nearby activities, etc.):

 \Box Condition of guard posts:

- □ Condition of surface seal, if visible. (e.g. concrete cracked, raised, , loose, etc.): ___________________________
- □ Condition of protective casing (e.g. extensive rust, broken hinge, loose): _________
- □ Well Security (locked/unlocked, lock broken, etc.): _
- □ Evidence of contamination (e.g., animal or insect parts, recently painted, etc.):
- □ Any obstruction or kinks in the well casing (observed while lowering pump): ________________________________
- Condition of well casing (e.g. intact, cracked, split, etc.): _________________________________
- \Box Observation or evidence from bailing/pumping of sediment in well:

Other Notes:

Appendix D – Example Chain of Custody Form

Eurofins TestAmerica, Cedar Falls

Chain of Custody Record

704 Enterprise Drive

<u></u> eurofins

Environment Testing
TestAmerica